

# PLANS OF PROPOSED P.P.C.C. BRIDGE OVER ON

**LENGTH** 24 368 OUT TO OUT OF ABUTMENT PRECAST BACKWALL PANELS

**SUPERSTRUCTURE** TWO SIMPLY SUPPORTED SPANS OF PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS WITH ASPHALT OVERLAY

**SUBSTRUCTURE** TWO PRECAST CONCRETE ABUTMENTS AND ONE INTERMEDIATE BENT WITH STEEL H-PILES

**ROADWAY WIDTH** 9 600 OUT TO OUT OF GIRDERS

**LOCATION** IN R.M. OF

## SHEET LEGEND

1. COVER SHEET
2. GENERAL ELEVATION
3. BORING LOGS
4. SITE AND EROSION CONTROL DETAILS
5. ASSEMBLY DETAILS
6. ASSEMBLY DETAILS
7. ICE BREAKER DETAILS
8. STEEL PILE CAP DETAILS
9. STEEL PILE CAP DETAILS
10. BEARING AND ERECTION DETAILS
11. RAILING LAYOUT AND DETAILS
12. RAILING DETAILS
13. RAILPOST DETAILS
- P1. PRECAST PANEL DETAILS
- P2. PRECAST PANEL DETAILS
- G1. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G2. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G3. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G4. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS
- G5. PRECAST PRESTRESSED CHANNEL GIRDER DETAILS

## DESIGN DATA

### SPECIFICATIONS

AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/97 Interims

### VEHICULAR LIVE LOADING

1. Modified AASHTO HSS-25 Truck
2. AASHTO LRFD "HL-93" Loading

### STRUCTURAL CONCRETE

CSA A23.1, Exposure Class C-1 Air content category 1

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS -  $f_c = 45$  MPa at 28 days  
 $f_{ci} = 35$  MPa at time of de-stressing
2. PRECAST PANELS -  $f_c = 35$  MPa

### REINFORCING STEEL

1. PRECAST PRESTRESSED CONCRETE CHANNEL GIRDERS - CAN/CSA-C30.18-M92 Grade 400W black (i.e no epoxy coating)
2. PRECAST PANELS - CAN/CSA-C30.18-M92 Grade 400W black (i.e no epoxy coating)

### STRUCTURAL STEEL

1. All Structural Steel shall conform to CAN/CSA G40.21-M92 Grade 300W
2. HSS Tubing for Bridge Rail shall conform to CAN/CSA G40.21-M92 Grade 350W

### PRESTRESSING STRAND

20-13 # low relaxation strands,  $f_{pu} = 1860$  MPa

### PILE LOADING

	END PILE BENTS	INTERMEDIATE PILE BENTS
MAXIMUM FACTORED LOAD	KN	KN
FACTORED BEARING RESISTANCE	KN	KN

## HYDRAULIC DESIGN DATA

### DESIGN DISCHARGE

Q3% = m<sup>3</sup>/sec  
V3% = m/s

## SURVEY CONTROL

HORIZONTAL DATUM: NAD83CSRS

VERTICAL DATUM: CGVD28

ELLIPSOID: GRS 1980

GEOID (HT2.0): \_\_\_\_\_

UTM: ZONE \_\_\_\_\_

SCALE FACTOR: \_\_\_\_\_

### SITE CONTROL POINT DATA

CONTROL POINT # _____	NORTHING: _____
	EASTING: _____
	ELEVATION: _____
	DATE: _____
CONTROL POINT # _____	NORTHING: _____
	EASTING: _____
	ELEVATION: _____
	DATE: _____
CONTROL POINT # _____	NORTHING: _____
	EASTING: _____
	ELEVATION: _____
	DATE: _____



TP. -

RGE. -

### LOCATION MAP

Not to Scale

# MANITOBA INFRASTRUCTURE

WATER MANAGEMENT AND STRUCTURES

RELEASED FOR CONSTRUCTION BY :

EXECUTIVE DIRECTOR OF STRUCTURES

DATE \_\_\_\_\_

### ENVIRONMENTAL APPROVALS

- MANITOBA ENVIRONMENT ACT LICENCE  
DATE : \_\_\_\_\_  
FILE # : \_\_\_\_\_
- FISHERIES AND OCEANS CANADA - AUTHORIZATION OR REVIEW  
DATE : \_\_\_\_\_  
FILE # : \_\_\_\_\_
- TRANSPORT CANADA - NAVIGATION ACT  
DATE : \_\_\_\_\_  
FILE # : \_\_\_\_\_
- MANITOBA INFRASTRUCTURE ENVIRONMENTAL APPROVAL  
DATE : \_\_\_\_\_  
FILE # : \_\_\_\_\_
- ENVIRONMENTAL REVIEW COMPLETED  
DATE : \_\_\_\_\_  
COMPLETED BY : \_\_\_\_\_

ALL DIMENSIONS ARE IN MILLIMETRES (mm) AND ALL ELEVATIONS AND STATIONS ARE IN METRES (m) UNLESS SHOWN OTHERWISE.

DRAWN BY:

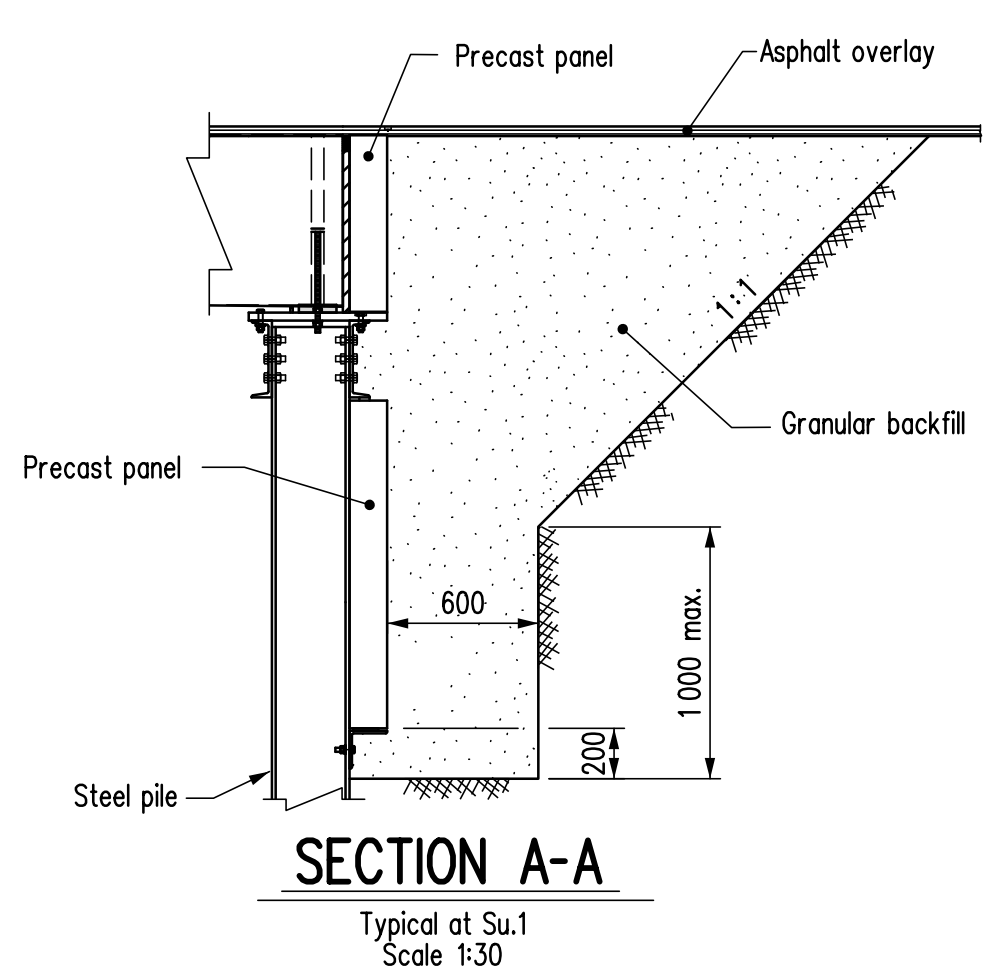
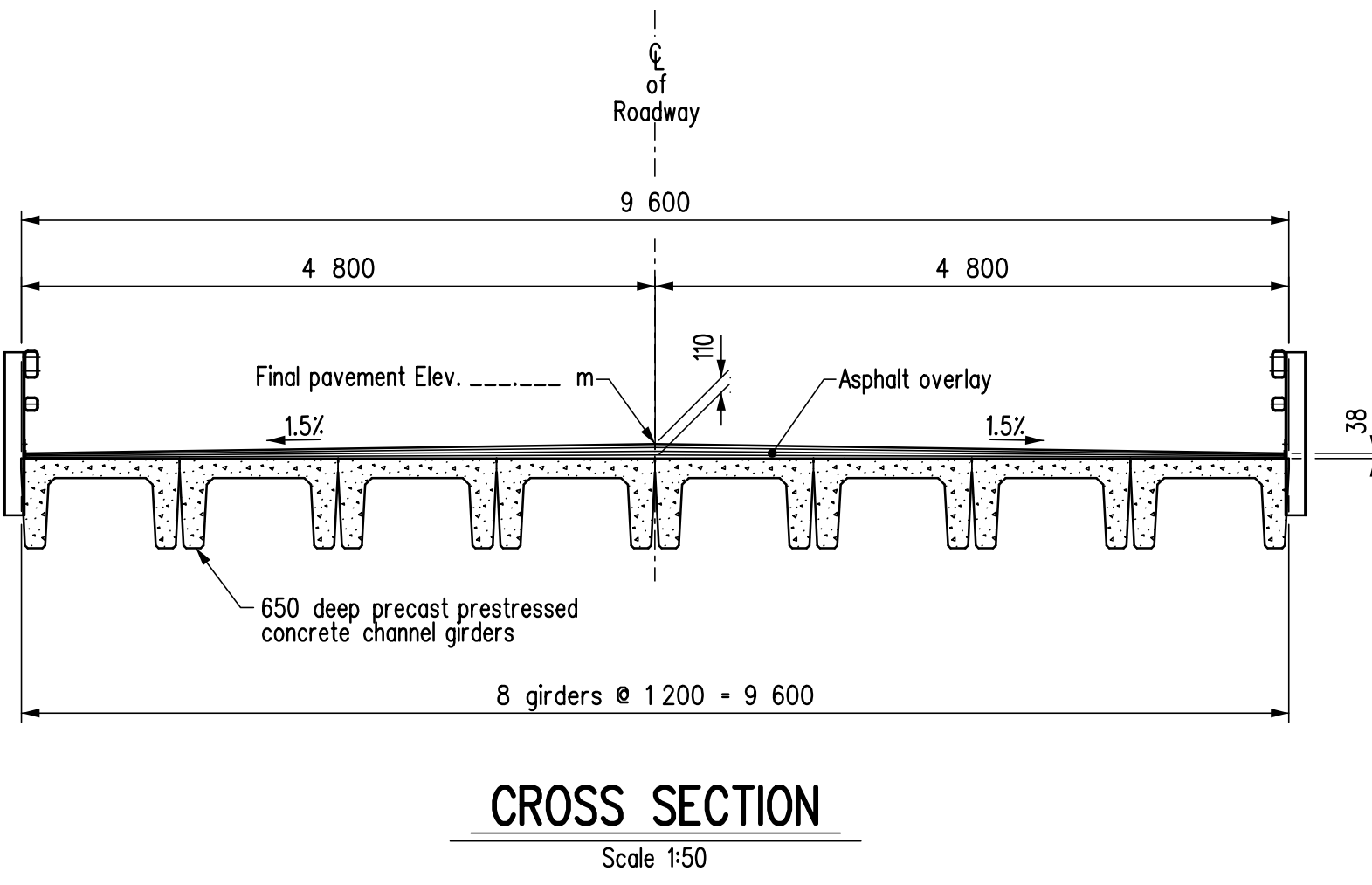
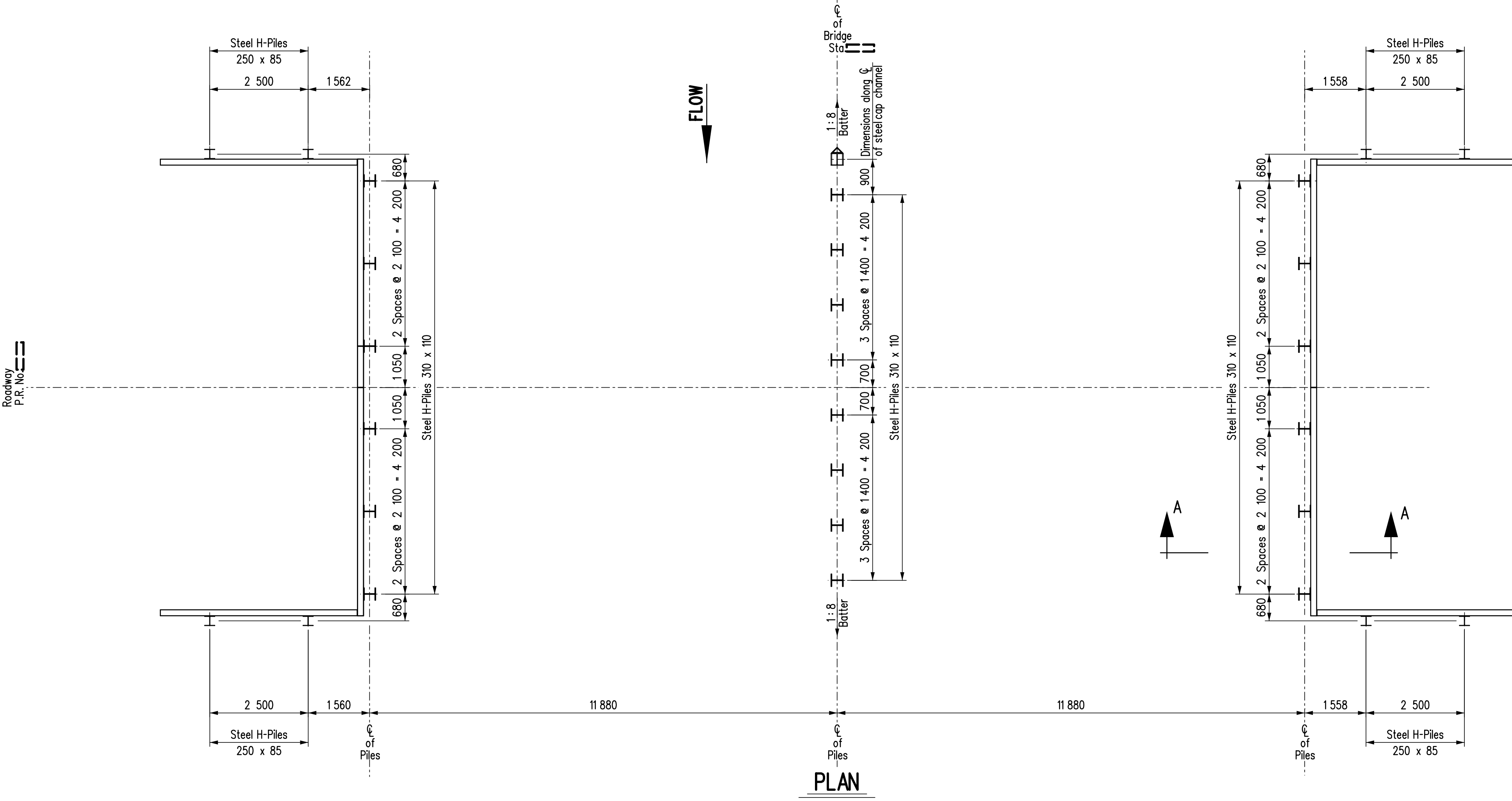
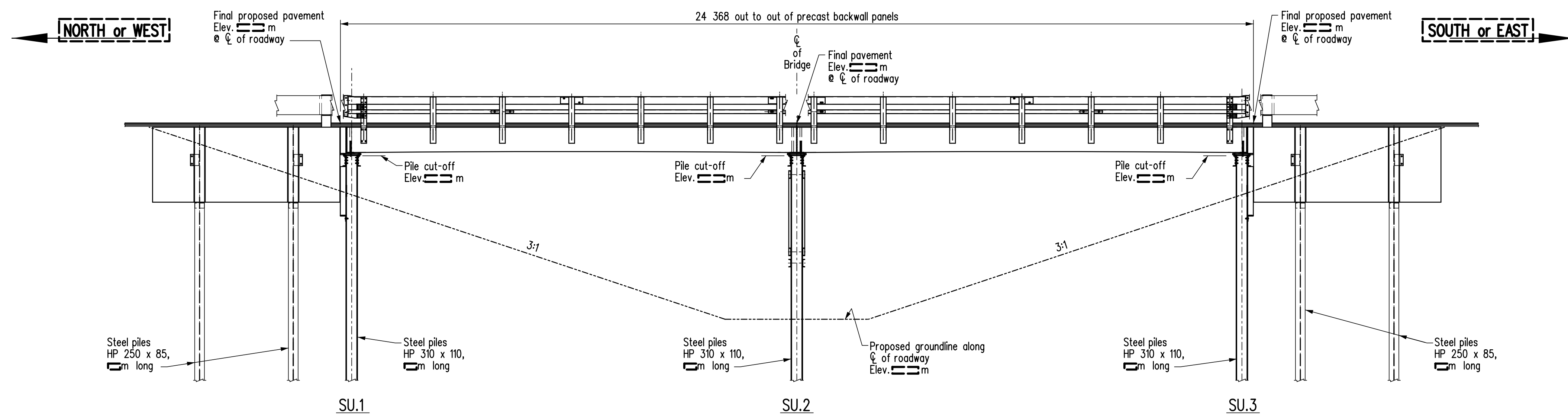
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SHEET No. 1

CHECKED BY:

DATE:

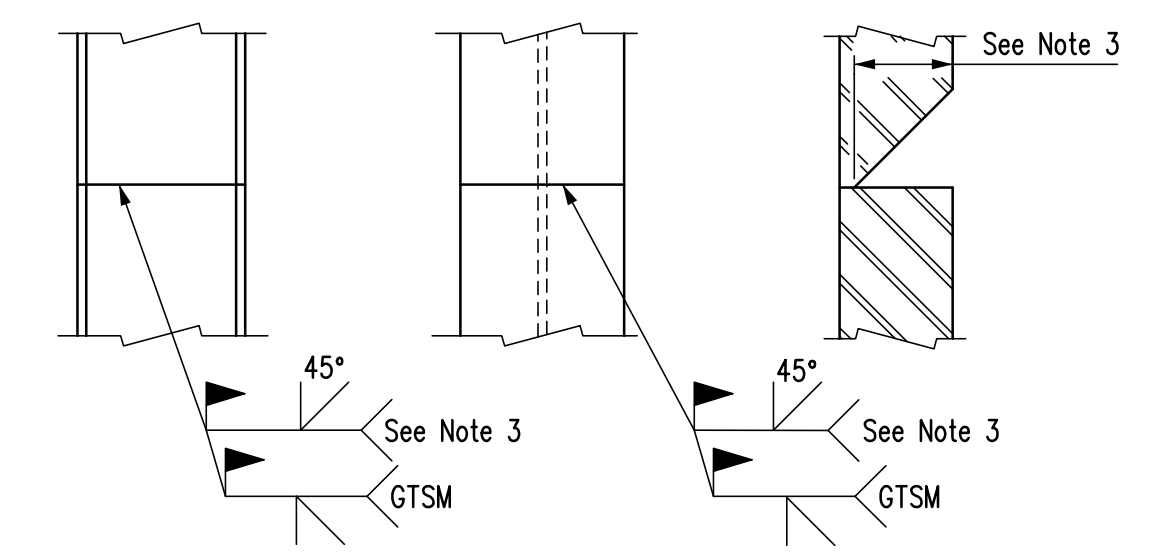
SITE No.



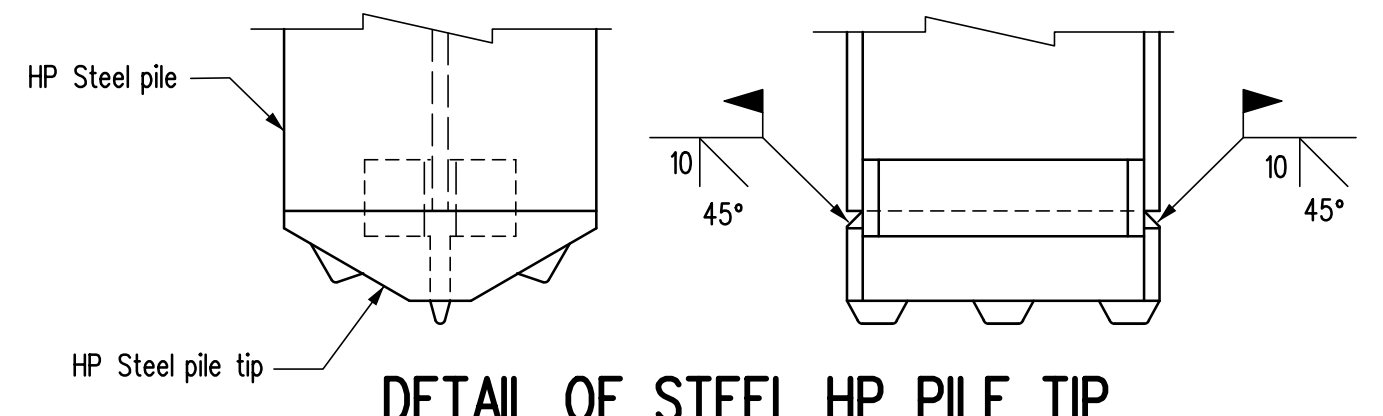
- NOTES:**  
re: Backfill Behind Abutment Ballast Walls
- Backfill behind ballast wall and wingwall panels shall be Type 1 - Granular backfill supplied and placed in accordance with Bridge Specification 1001 (I). The granular backfill shall be placed and compacted in lifts not exceeding 150 mm.
  - Compaction equipment used within 2 m of ballast walls and wingwalls shall be limited to light vibratory equipment with a mass not exceeding 120 kg unless otherwise approved.
  - Steel pile tip to be PRUYN "Hard-Bite" or equivalent.

BILL OF PILES		Site No.	
LOCATION	DESCRIPTION	No. OF PILES	TOTAL LENGTH (m)
SU.1 & SU.3	Steel piles - HP310 x 110 (abutments)	12	0
SU.1 & SU.3	Steel piles - HP250 x 85 (wing walls)	8	0
SU.2	Steel piles - HP310 x 110 (Intermediate bent)	8	0
SU.2	Steel piles - HP310 x 110 (Intermediate bent) - Ice Breaker Pile	1	0
TOTAL LENGTH OF PILES (m) =		0	0

BILL OF PILE TIPS		No. OF PILES
LOCATION	DESCRIPTION	No. OF PILES
SU.1 & SU.3	Hard-Bite Point HP-77750-B for HP310 x 110 (Abutments)	12
SU.2	Hard-Bite Point HP-77750-B for HP310 x 110 (Intermediate bent) - Excluding Ice Breaker Pile	8



- DETAIL OF STEEL HP PILE SPLICE**  
Not To Scale
- NOTES:**  
re: Welding
- Low hydrogen E70 series electrodes shall be used.
  - The minimum root pass shall be 6 mm.
  - Preparation for welding requires 13 mm bevel for HP 250 piles and 14 mm bevel for HP 310 piles.
  - Weld both flanges and web as shown. The inside beveling and welds to be completed first.
  - Before undertaking the back welds, the weld preparation shall be carried out with a carbon Arc-Air gouger.

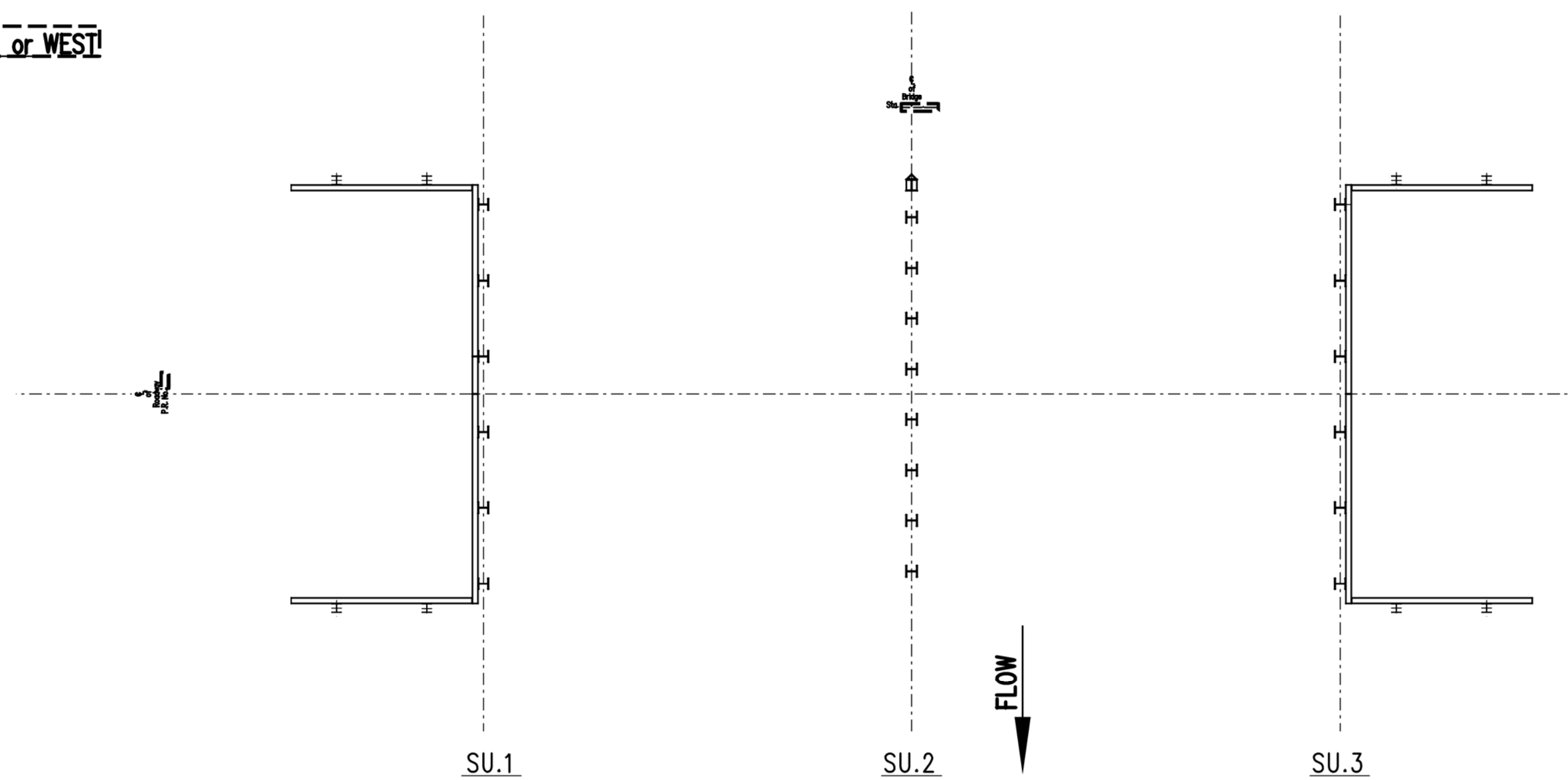


- DETAIL OF STEEL HP PILE TIP**  
Not To Scale
- NOTES:**
- Edges of HP Steel pile tip to be ground on 45° bevel for 10 mm.
  - Low hydrogen E70 series electrodes shall be used.
  - The minimum root pass shall be 6 mm.
- E48018 equivalent metric electrode

REVISIONS		GENERAL ELEVATION	
DATE	BY	DESIGN SEAL	RECORD SEAL
<p><b>PLACE ENGINEERS ELECTRONIC SEAL HERE</b></p>			
		<p>EXECUTIVE DIRECTOR OF STRUCTURES DATE</p> <p>SCALE: 1:75 SHEET No. 2</p> <p>or as shown SITE No. 2</p>	

← NORTH or WEST

→ SOUTH or EAST



**PLAN**  
Showing Bore Hole locations

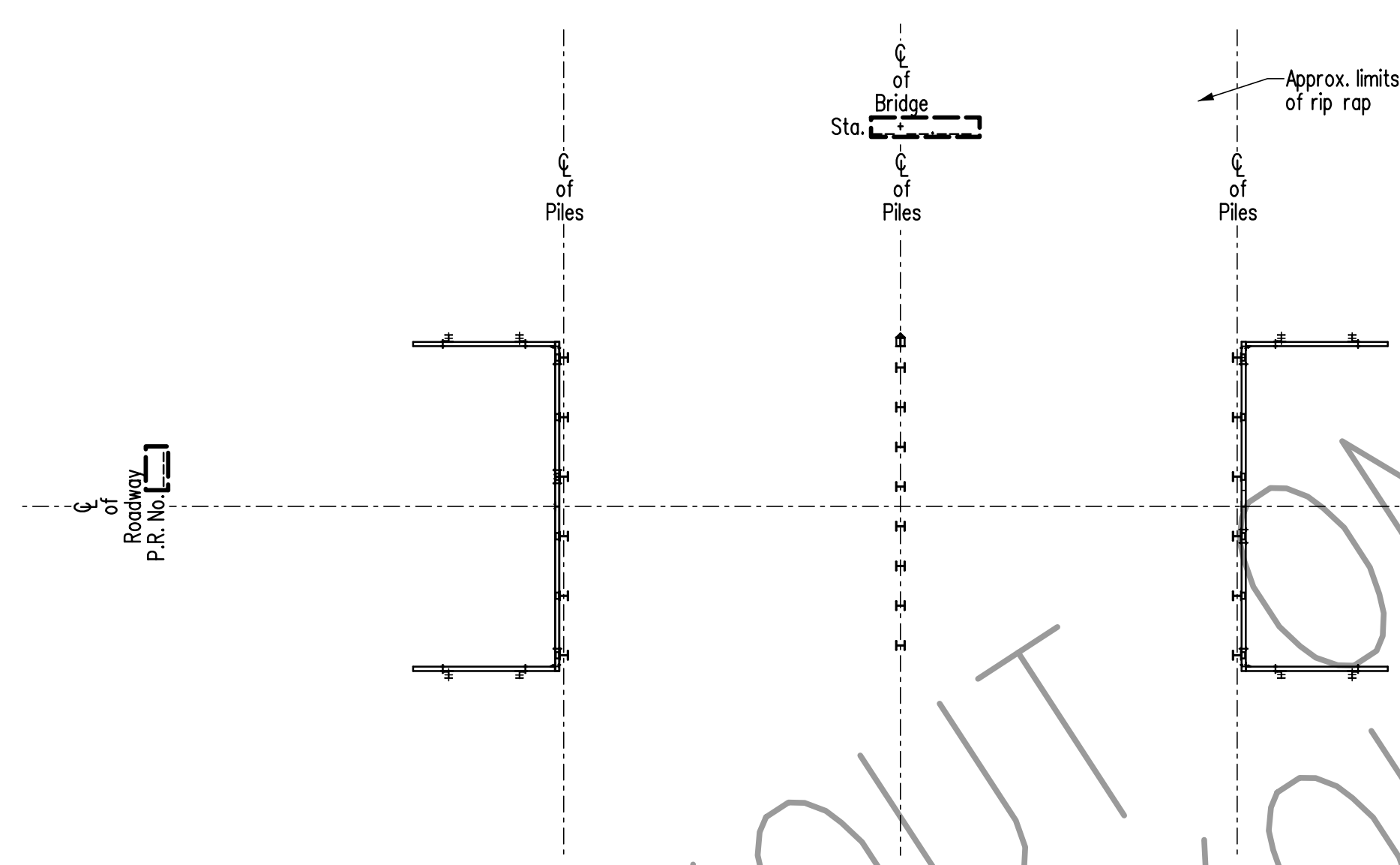
**NOTES - re: Boring Logs**

- The Department provides log boring information shown on the Plans. This information may not be representative of the soil conditions throughout the site. Contractors may peruse all available soil information in the Water Management and Structures Branch located at 6th floor, 215 Garry Street, Winnipeg.
- The following abbreviations apply to bore hole information:
  - Qu - Laboratory unconfined compressive strength in kPa
  - SPT (N) - Number of blows per 300 mm - Standard Penetration Test
  - USC - Unified Soil Classification
  - M.C. - Moisture Content
- All stations, elevations, offsets and depths as shown are in meters. All dimensions are in millimeters.
- All bore hole locations shown in plan view are approximate.
- Elevations on boring logs are at a vertical scale of 1:100.

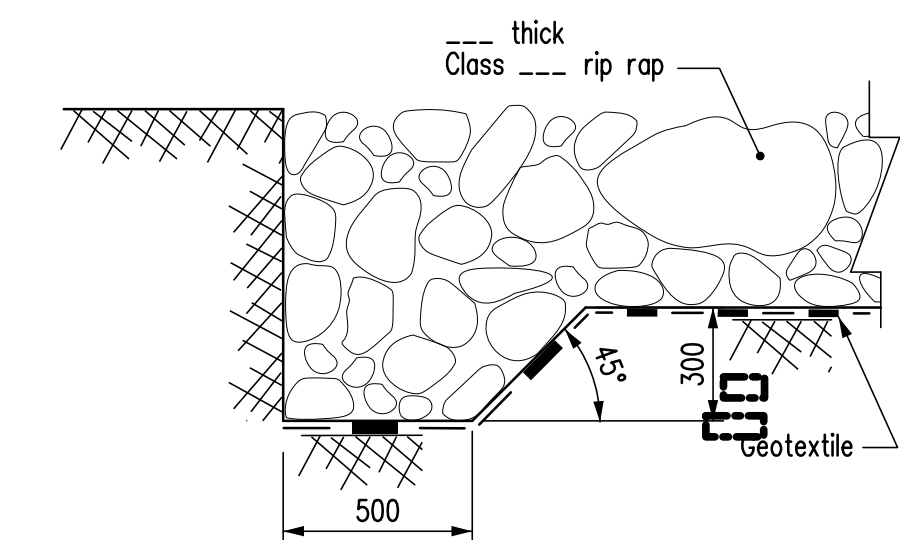
REVISIONS		BORING LOGS	
DATE	BY	DESCRIPTION	
		DESIGN SEAL	RECORD SEAL
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		BY: _____ CHECKED: _____	RELEASED FOR CONSTRUCTION BY: _____ EXECUTIVE DIRECTOR OF STRUCTURES DATE: _____
		BY: _____ CHECKED: _____	SCALE: 1:100 SHEET No. 3 or as shown SITE No. _____

NORTH or WEST

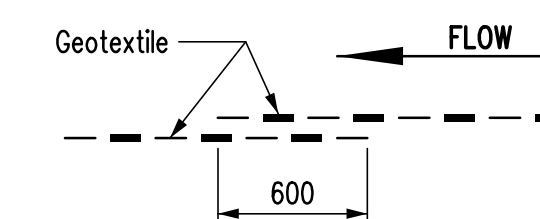
SOUTH or EAST



PLAN



EDGE TREATMENT



OVERLAPPING DETAILS

RIP RAP DETAILS

Not To Scale

NOTES:

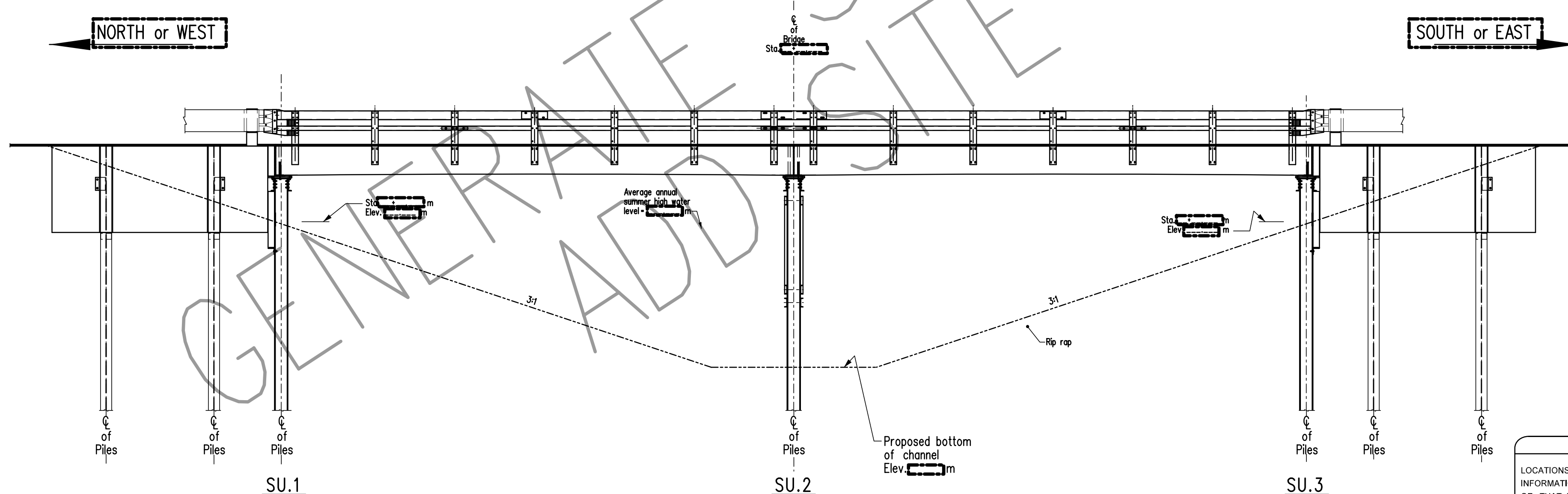
1. All geotextile shall be Non-Woven Geotextile, Class I (Heavy Duty) from the Manitoba Infrastructure's Approved Product List.
2. Geotextile shall be placed under all rip rap, overlapping 600mm in direction of flow.

NOTE:

Existing pile bents to be removed by Bridge Contractor.

NORTH or WEST

SOUTH or EAST



SU.1

SU.2

SU.3

ELEVATION

Scale 1:75

UTILITY DISCLAIMER:

LOCATIONS OF UTILITIES AS SHOWN ARE BASED ON READILY AVAILABLE INFORMATION. NO GUARANTEE IS GIVEN THAT ALL UTILITIES ARE SHOWN OR THAT THE GIVEN LOCATIONS ARE EXACT. CONTRACTOR SHALL CONFIRM THE EXISTENCE AND LOCATION OF UTILITIES BY OBTAINING FROM THE INDIVIDUAL UTILITIES BEFORE PROCEEDING WITH CONSTRUCTION.

REVISIONS

DATE	BY	DESCRIPTION

PLACE ENGINEERS  
ELECTRONIC SEAL  
HERE

SITE AND EROSION CONTROL DETAILS

**Manitoba**  
Infrastructure  
Water Management and Structures

RELEASED FOR CONSTRUCTION BY:

EXECUTIVE DIRECTOR OF STRUCTURES DATE

DESIGN

BY: \_\_\_\_\_  
CHECKED: \_\_\_\_\_

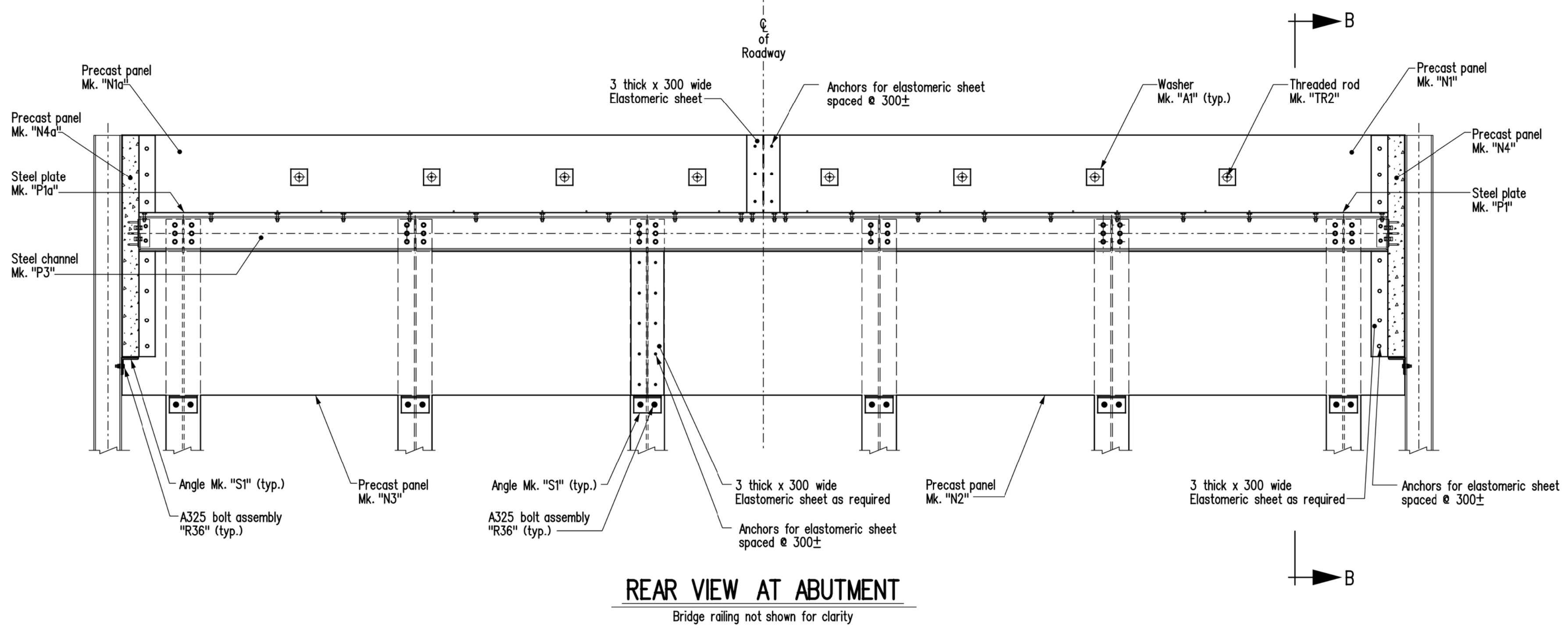
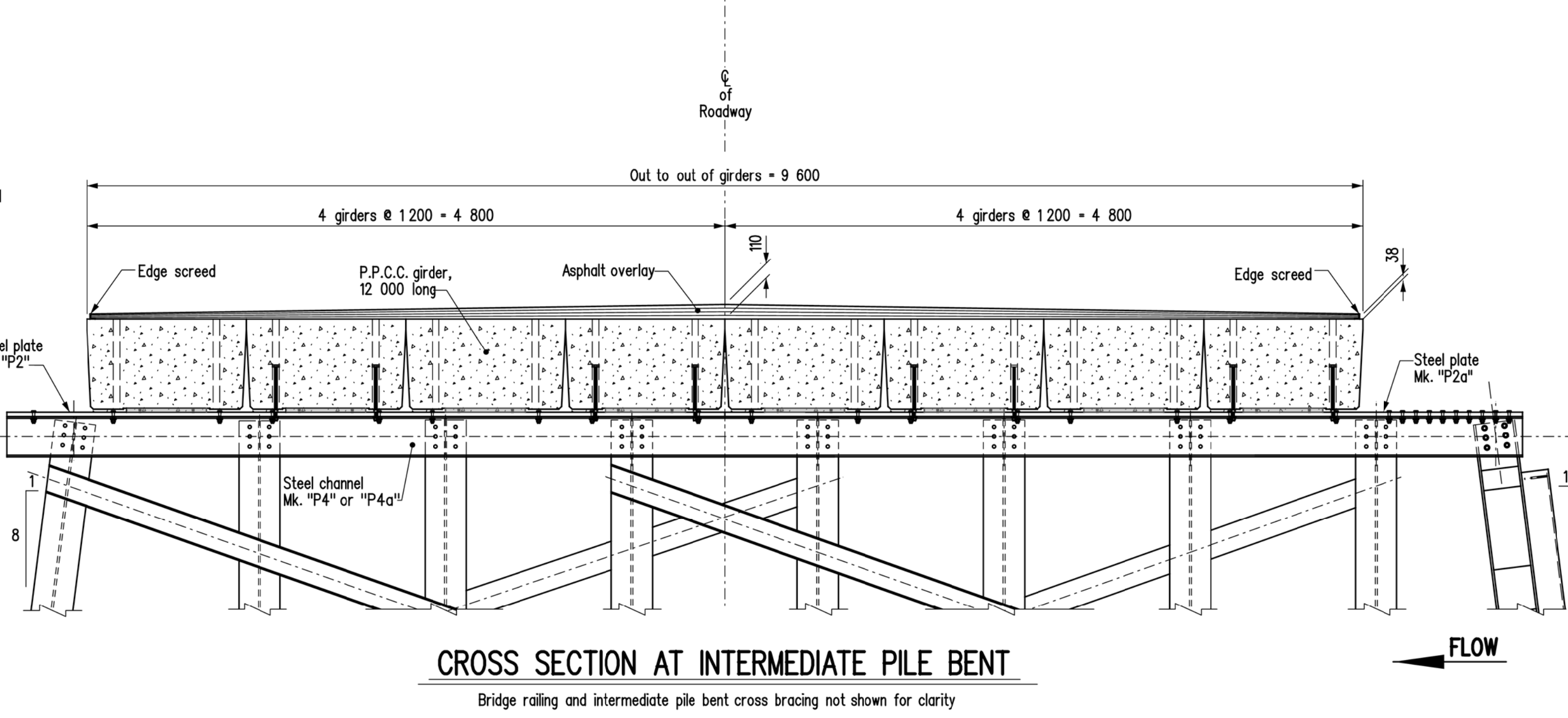
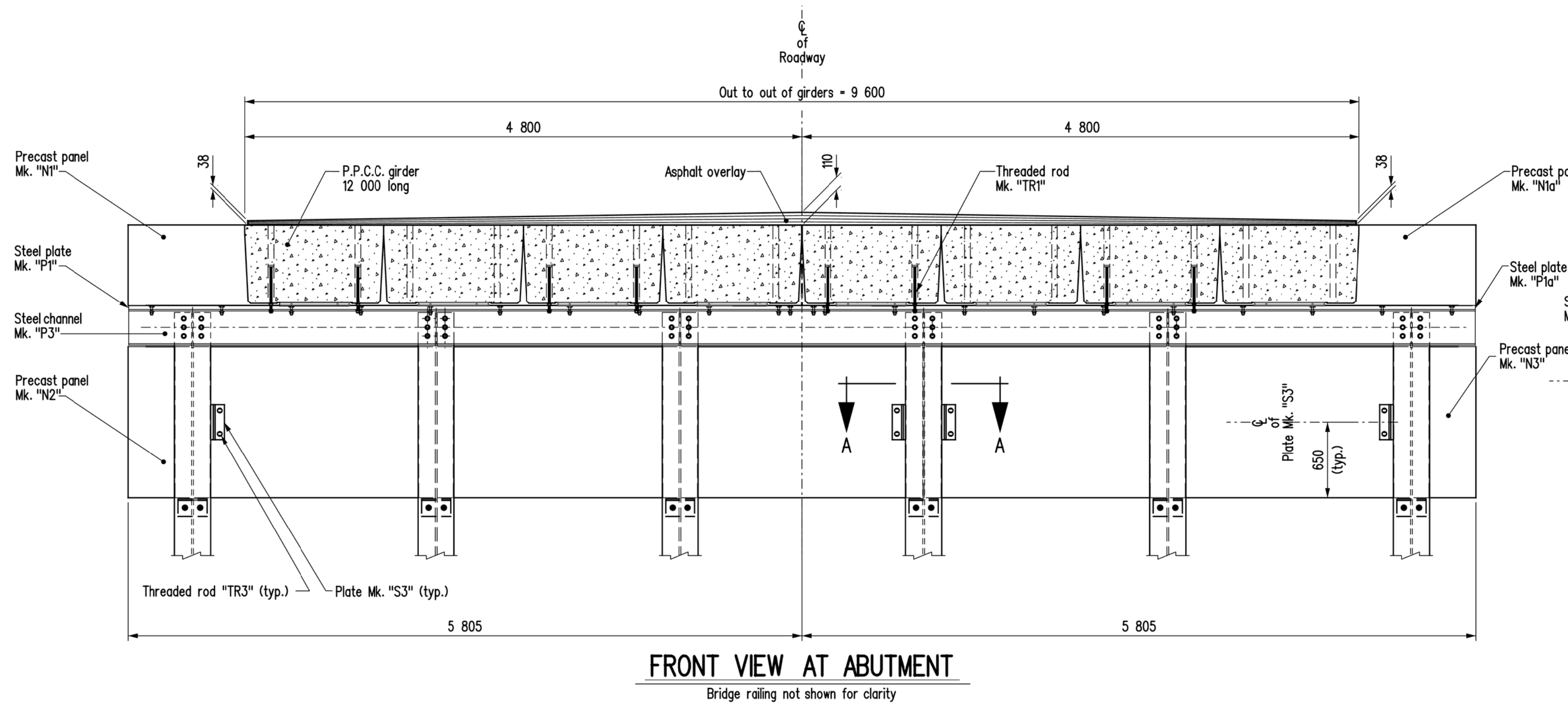
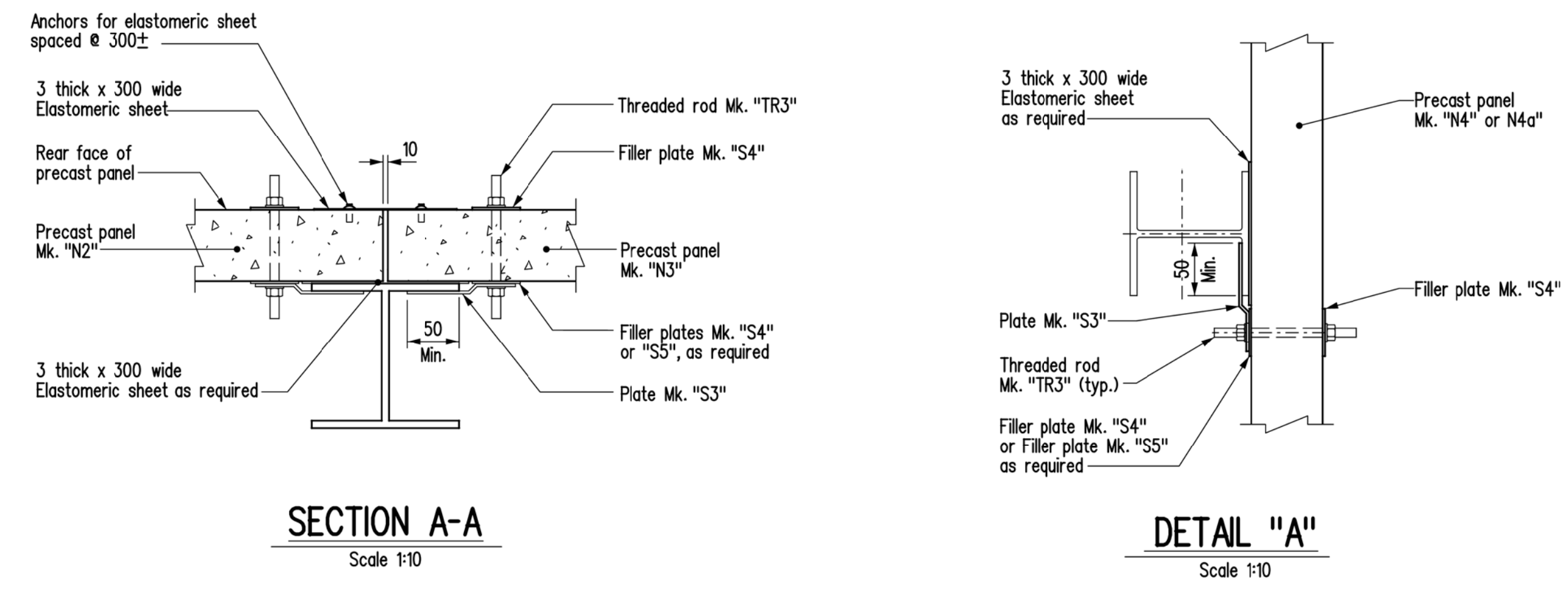
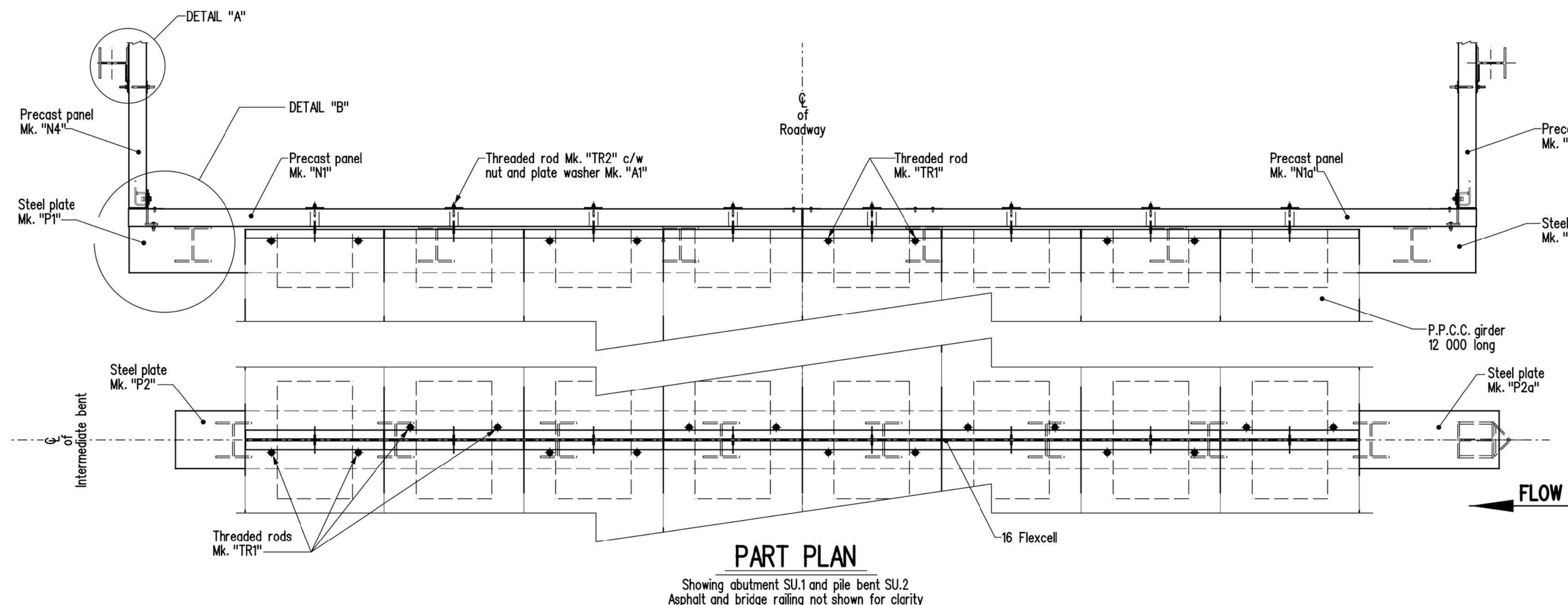
DETAILS

BY: \_\_\_\_\_  
CHECKED: \_\_\_\_\_

SCALE:

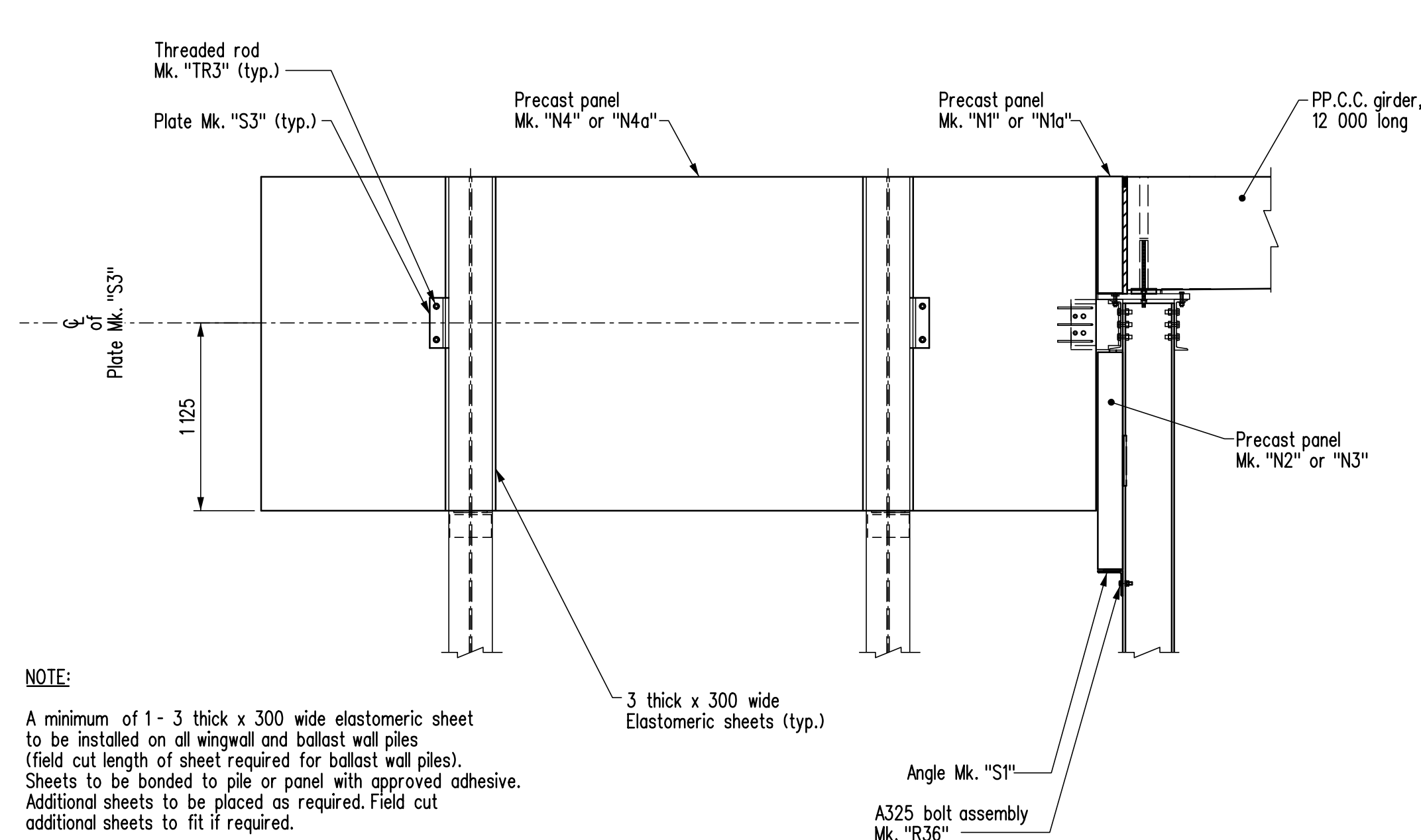
1:200 SHEET No. 4

or as shown SITE No. \_\_\_\_\_



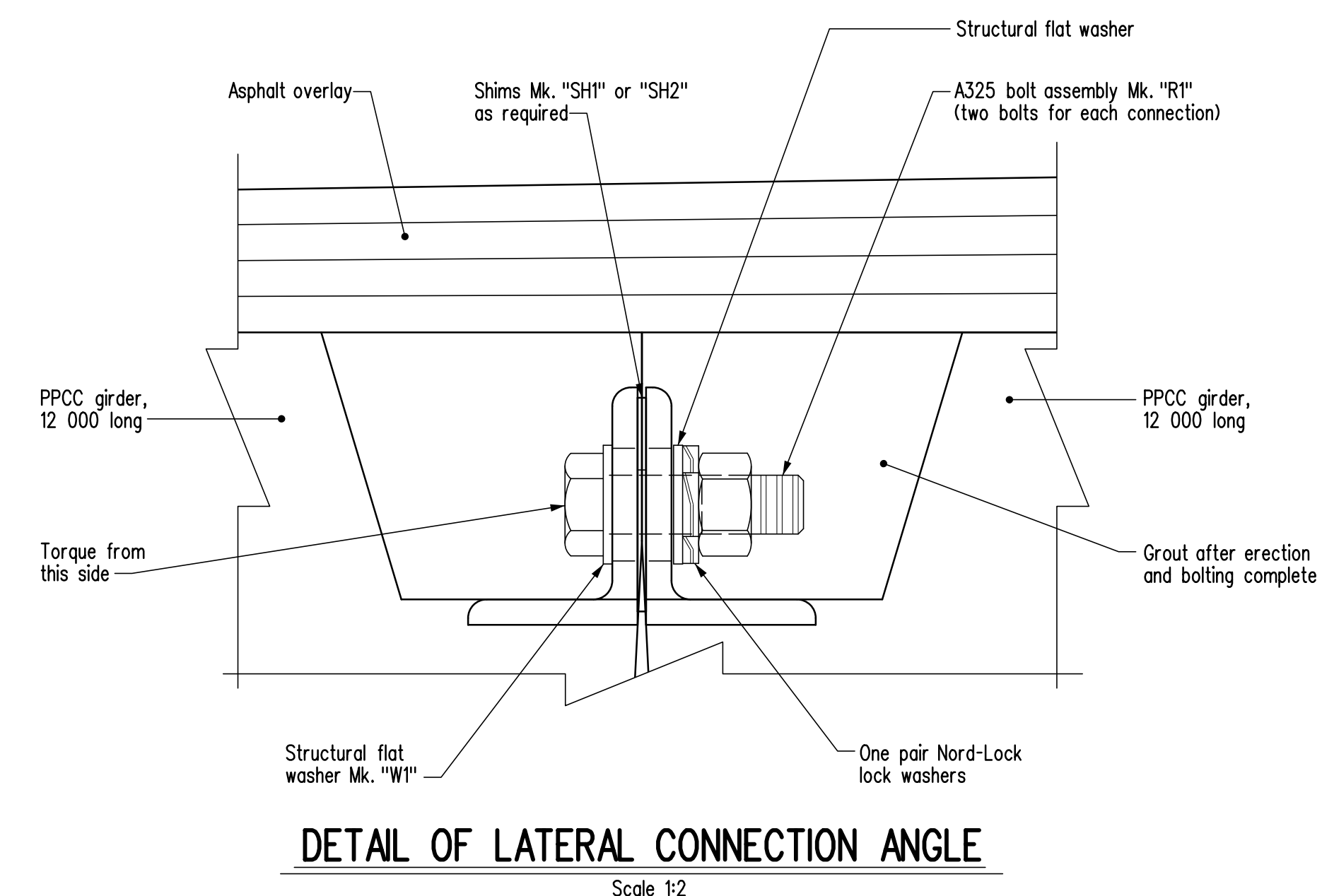
- NOTES:**
- For Section "B-B" and DETAIL "B" see Sheet No. [ ]
  - For "BILL OF MISCELLANEOUS METAL" see Sheet No. [ ]
  - The Contractor shall field drill 22 # holes in the precast panels for threaded rods Mk. "TR3". Should rebar be encountered, abandon hole, patch and drill in new location. Rebar locations are marked on the panels by the Panel Fabricator.
  - Back faces of the upper and lower ballast walls shall be aligned in the same vertical plane.
  - The Contractor shall ensure that the upper ballast walls are placed with the edge 5mm from  $\phi$  of roadway.

REVISIONS		ASSEMBLY DETAILS	
DATE	BY	DESIGN	RELEASED FOR CONSTRUCTION BY:
DESIGN SEAL	RECORD SEAL	EXECUTIVE DIRECTOR OF STRUCTURES	DATE
PLACE ENGINEERS ELECTRONIC SEAL HERE		BY: [ ]	SCALE: 1:30
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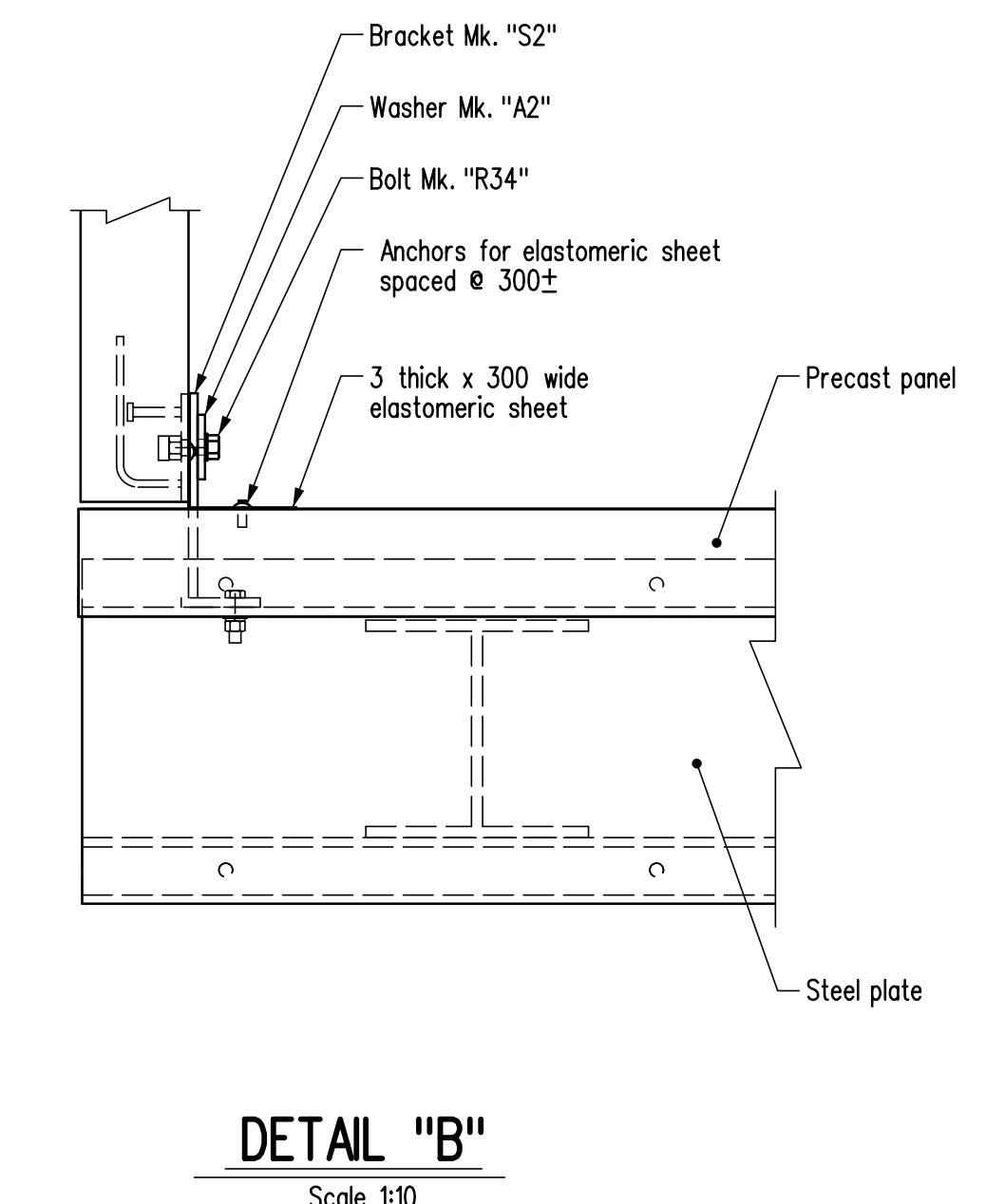


NOTE:  
A minimum of 1- 3 thick x 300 wide elastomeric sheet to be installed on all wingwall and ballast wall piles (field cut length of sheet required for ballast wall piles). Sheets to be bonded to pile or panel with approved adhesive. Additional sheets to be placed as required. Field cut additional sheets to fit if required.

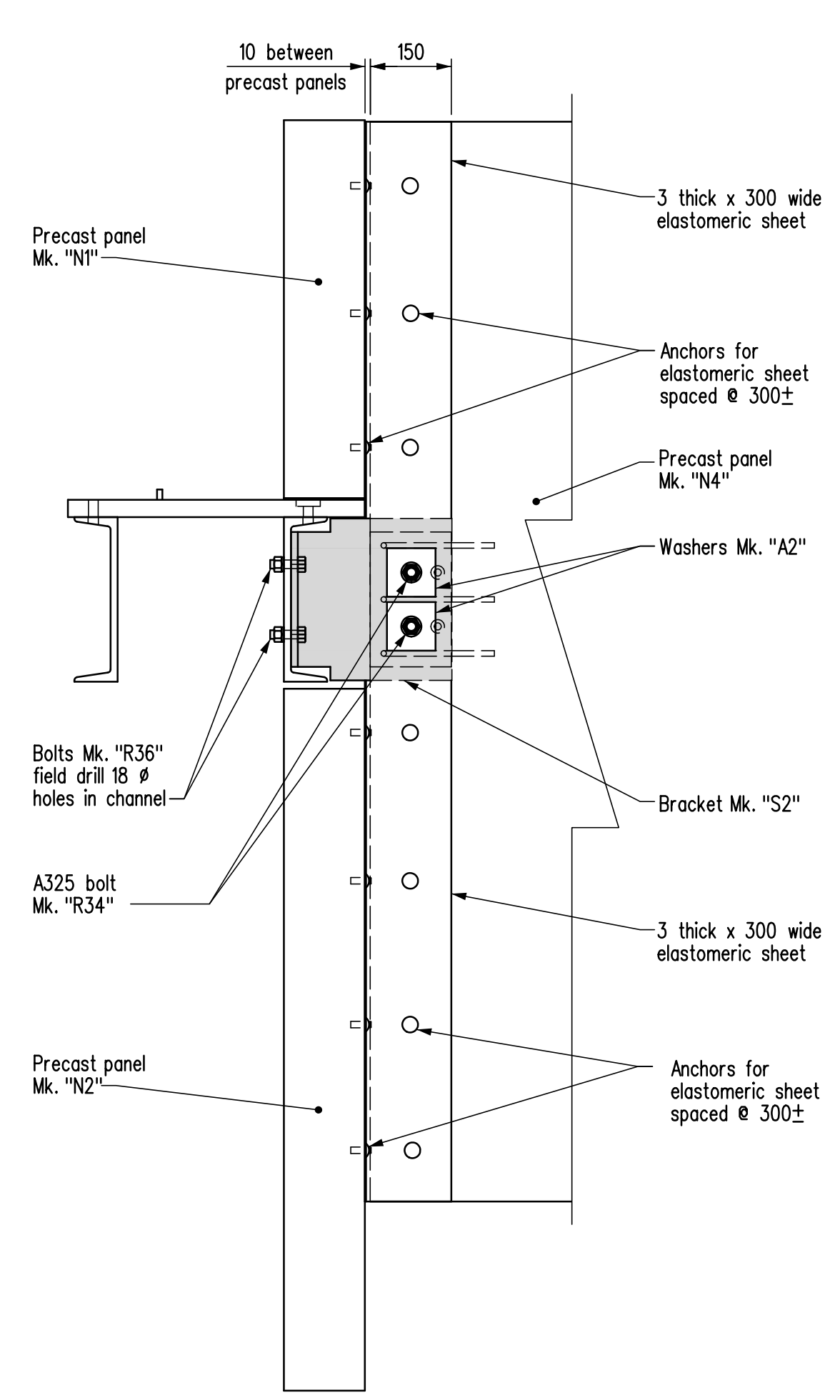
**PART SIDE ELEVATION**  
Bridge railing not shown for clarity



**DETAIL OF LATERAL CONNECTION ANGLE**  
Scale 1:2



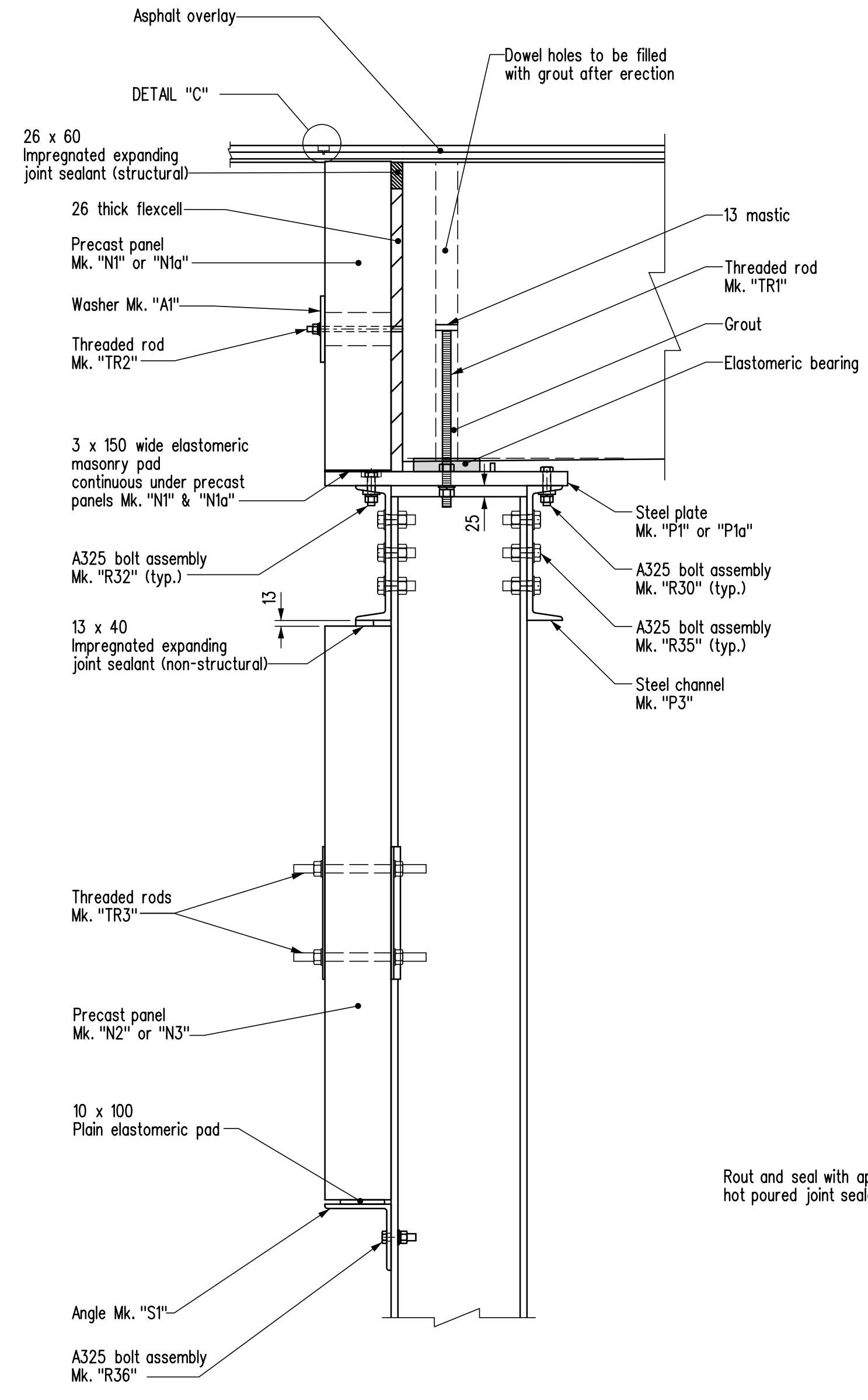
**DETAIL "B"**  
Scale 1:10



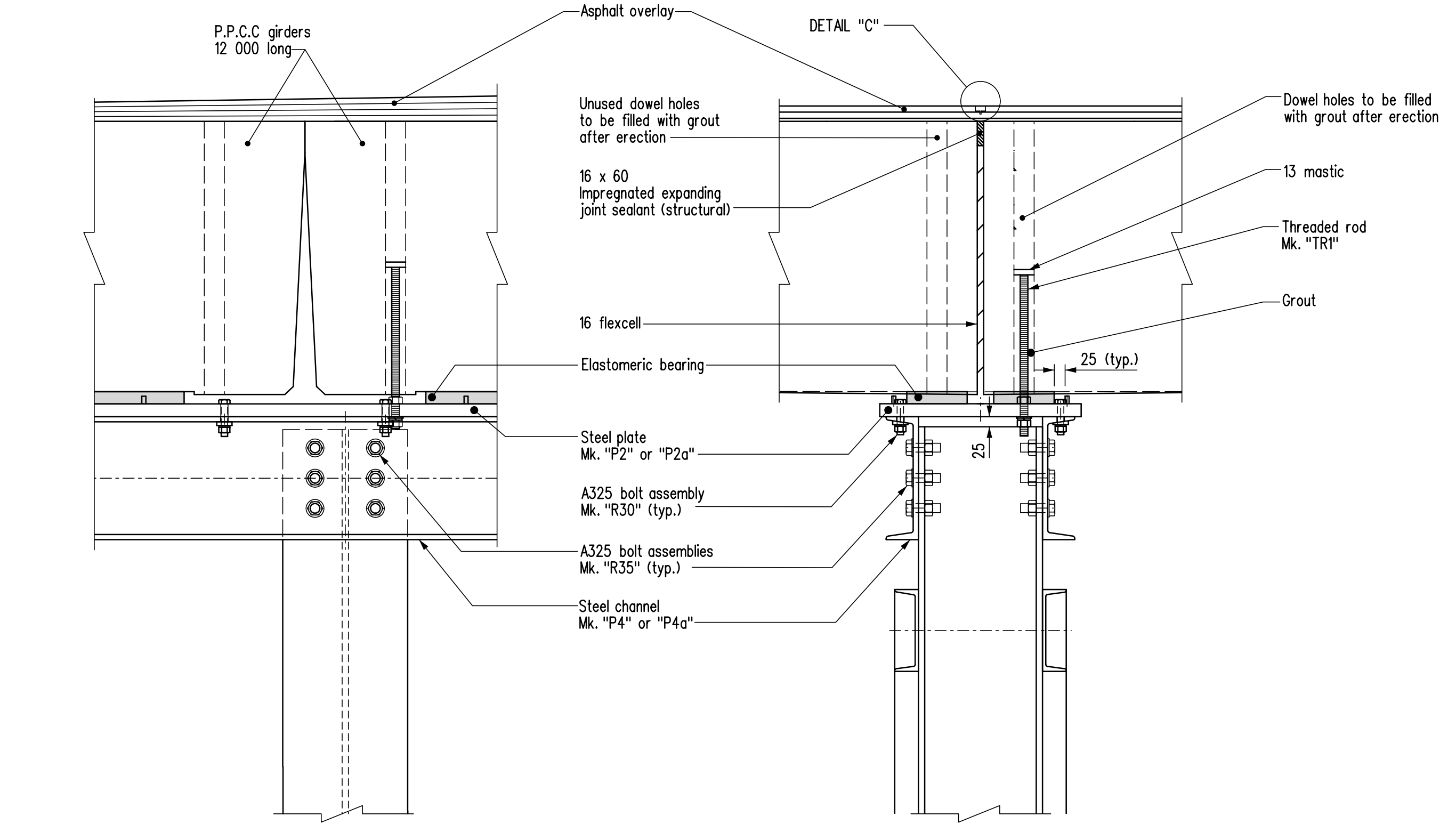
**SECTION B-B**  
Scale 1:10

NOTES:

- RE-BOLTING
  - GIRDER LATERAL CONNECTION
    - Bolts Mk. "R1" - c/w one F436 hardened washer, one structural plate washer Mk. "W1", one pair Nord-Lock washers and one Grade DH heavy hex. nut.
  - STEEL CAP
    - Bolts Mk. "R30" - One F436 hardened washer, one hardened bevel washer and one Grade DH heavy hex. nut.
    - Bolts Mk. "R32" - One hardened bevel washer and one Grade DH heavy hex. nut.
    - Bolts Mk. "R35" - Two F436 hardened washers and one Grade DH heavy hex. nut.
  - PRECAST PANELS
    - Bolts Mk. "R36" - Two F436 hardened washers and one Grade DH heavy hex. nut.
    - Bolts Mk. "R34" - One F436 hardened washer and one structural plate washer Mk. "A2", no nuts.
    - Threaded rod Mk. "TR2" - One standard flat washer, one structural lock washer, structural plate washer Mk. "A1" and one stainless steel hex. nut.
    - Threaded rod Mk. "TR3" - two Filler plates Mk. "S4", one structural lock washer, two standard flat washers and two hex. nuts, Filler plate Mk. "S5" if required.
  - GIRDER TO STEEL CAP
    - Threaded rod Mk. "TR1" - one standard flat washer and structural lock washer and two hex. nuts.
  - High strength bolts shall be tightened by the turn-of-nut method as per Bridge Specifications. Ensure nuts are lubricated prior to bolting.
  - Fill counter bored holes with mastic filler after tightening bolts.
- When grouting dowl holes in girders, ensure that there is no grout between bottom of girder and bearing plate.
- Apply galvalloy to all field welds & areas where galvanizing has been damaged.
- Impregnated expanding joint sealant shall be installed as per manufacturer's recommendations.

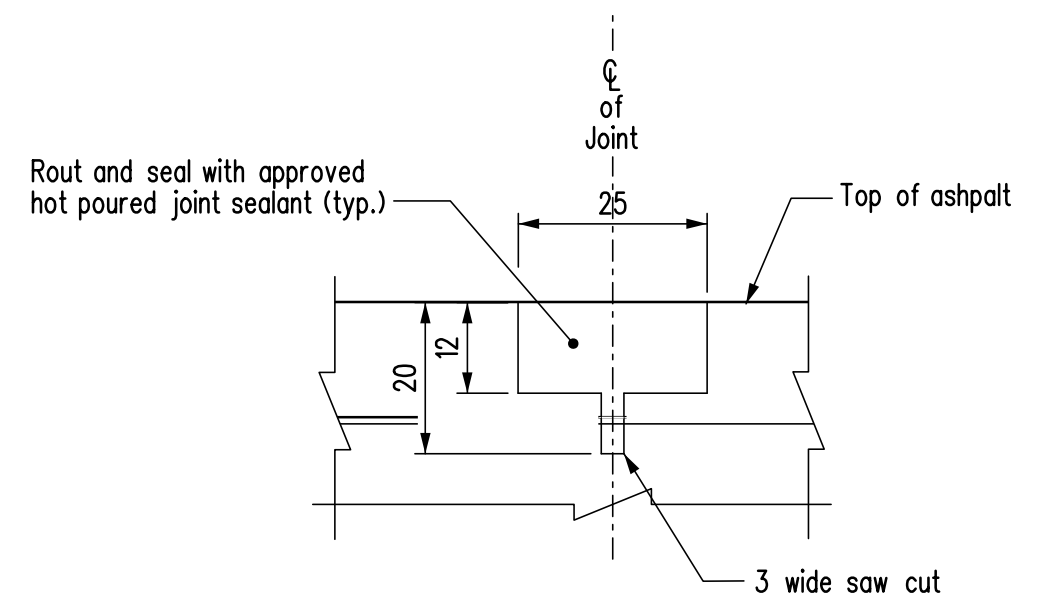


**SECTION AT ABUTMENT**  
Scale 1:10



**CROSS SECTION** **ELEVATION**

**DETAILS AT INTERMEDIATE BENT**  
Scale 1:10



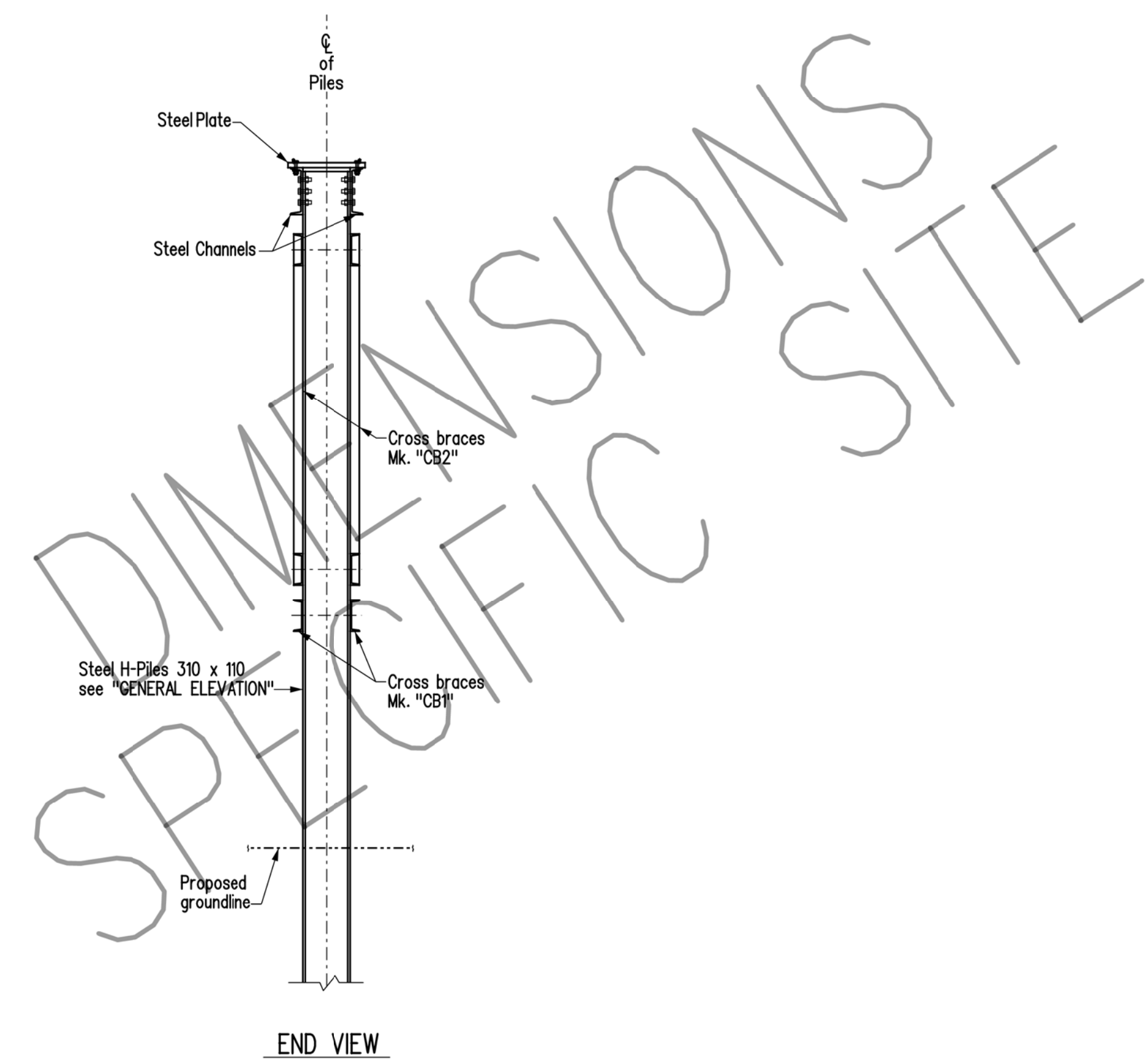
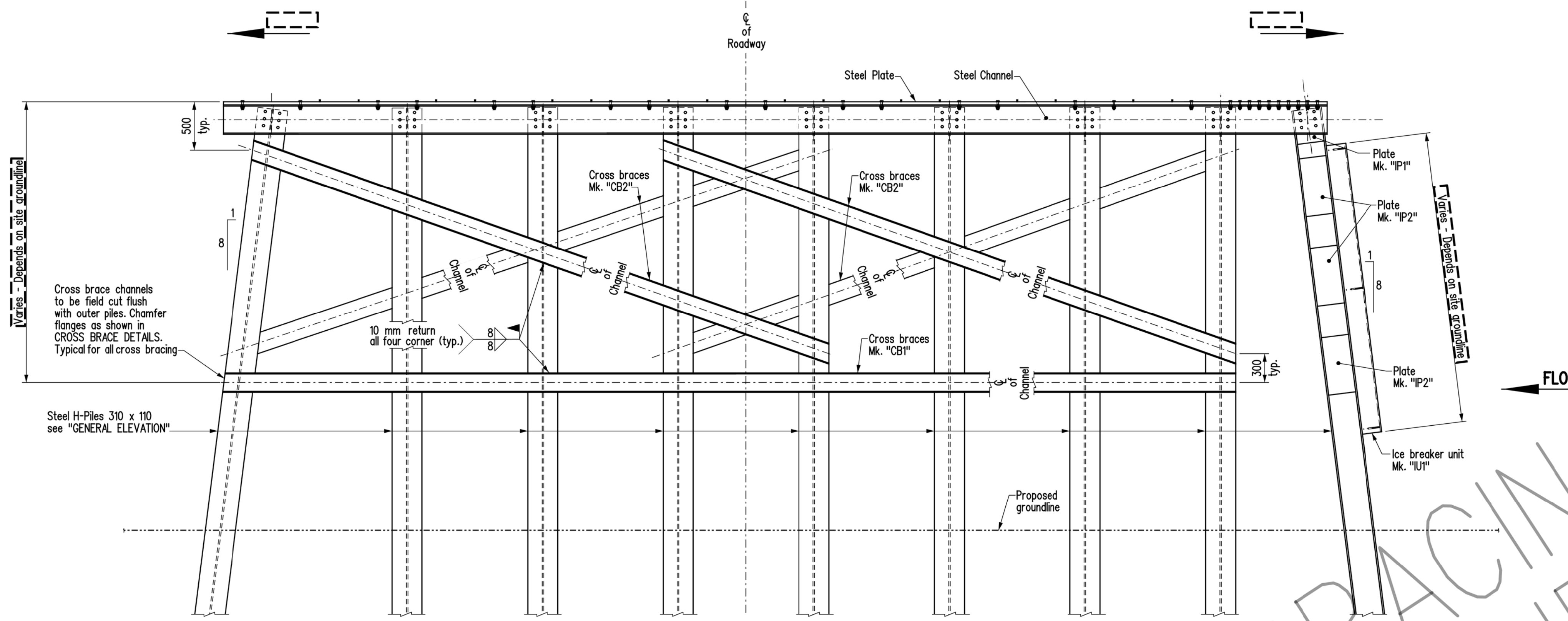
**DETAIL "C"**  
Scale 1:1

NOTE:  
For location of SECTIONS "B-B" & "DETAIL B" see Sheet No. 6.

REVISIONS		ASSEMBLY DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:

<b>Manitoba</b> Infrastructure Water Management and Structures	EXECUTIVE DIRECTOR OF STRUCTURES DATE	
	DESIGN	SCALE: 1:30 SHEET No. 7
	DETAILS	or as shown SITE No.

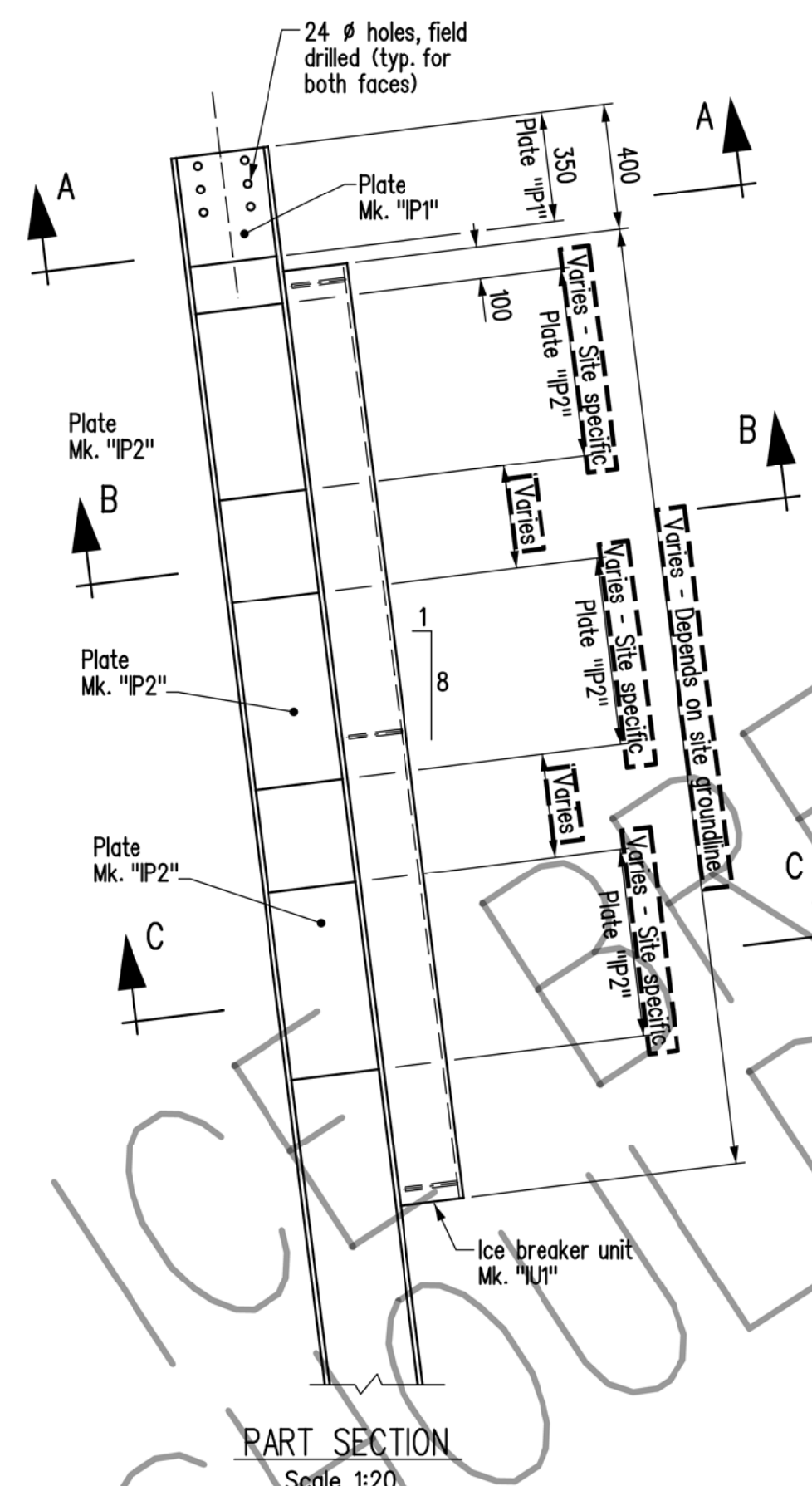


**NOTE: Re: CROSS BRACING AND ICE BREAKER COMPONENTS**

All cross bracing and ice breaker components shown shall be installed as per details, however if the river water level/ice level at the time of installation of bracing is such as to interfere with these components, the Contractor shall adjust as directed by Engineer.

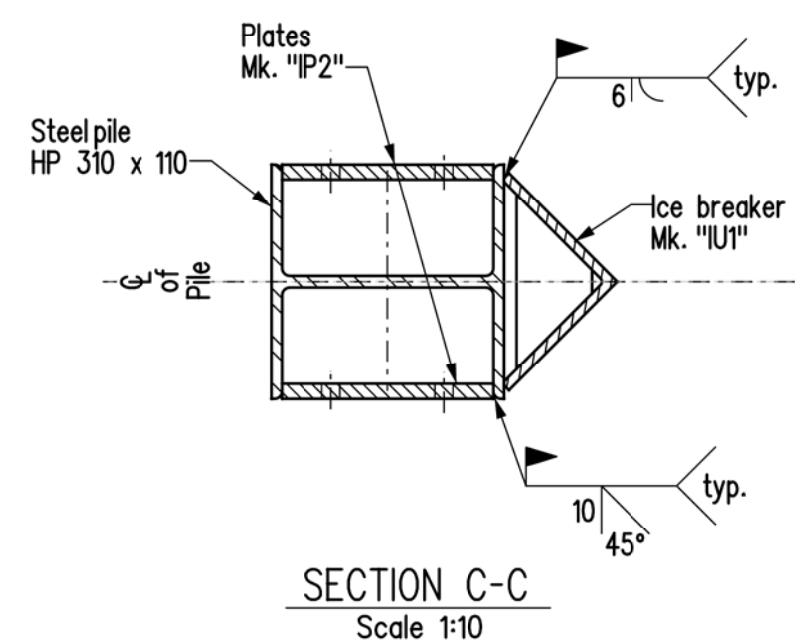
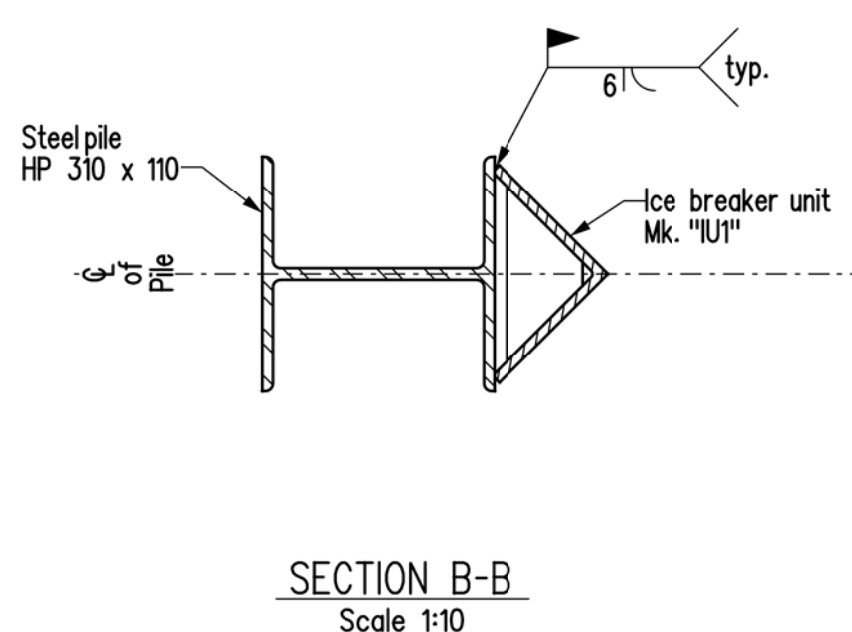
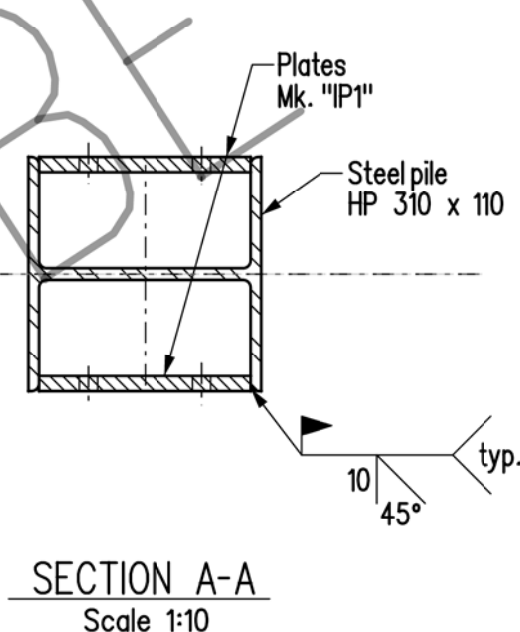
**CROSS SECTION**  
**INTERMEDIATE PILE BENT SU.2**

Showing intermediate pile bent SU.2  
Bridge superstructure not shown for clarity



**ICE BREAKER ASSEMBLY DETAILS**

Showing SU.2 ice breaker  
Steel plate and channel not shown for clarity



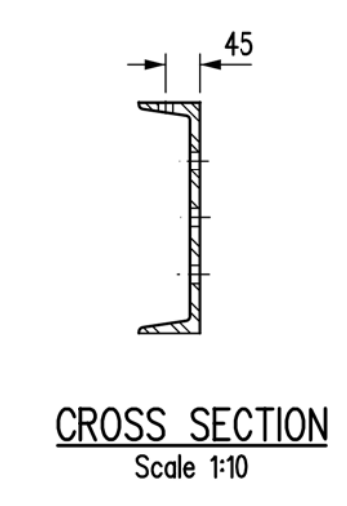
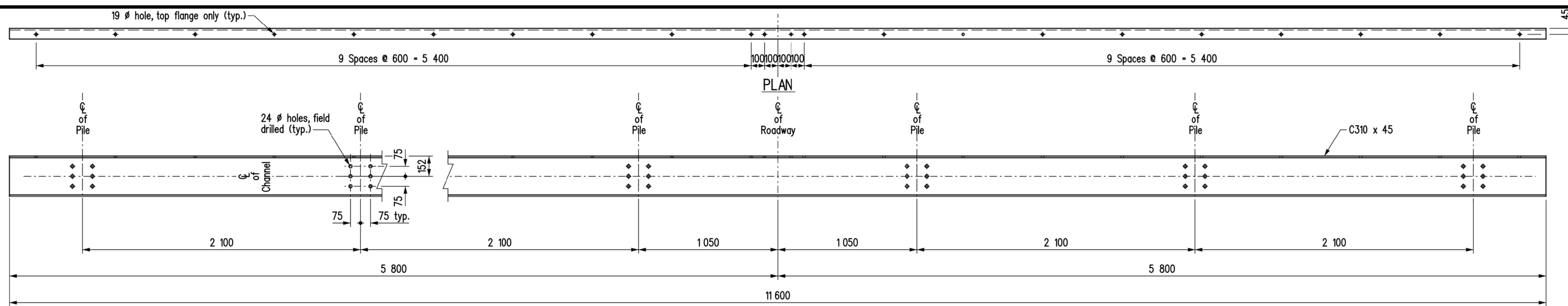
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DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION
			BY:
			EXECUTIVE DIRECTOR OF STRUCTURES
			DATE
			SCALE:
			1 : 30
			SHEET No. <u>8</u>
			or as shown
			SITE No. <u> </u>

PLACE ENGINEERS  
ELECTRONIC SEAL  
HERE

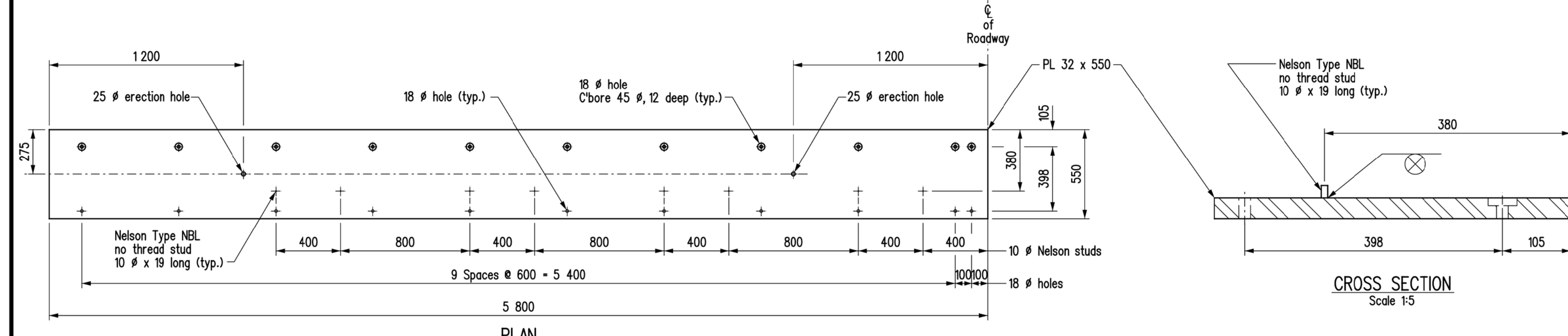


DESIGN BY:    
CHECKED:  

DETAILS BY:    
CHECKED:



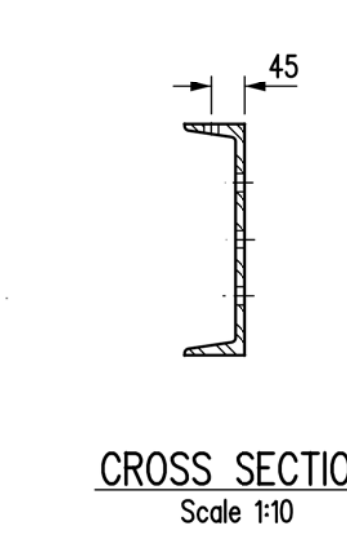
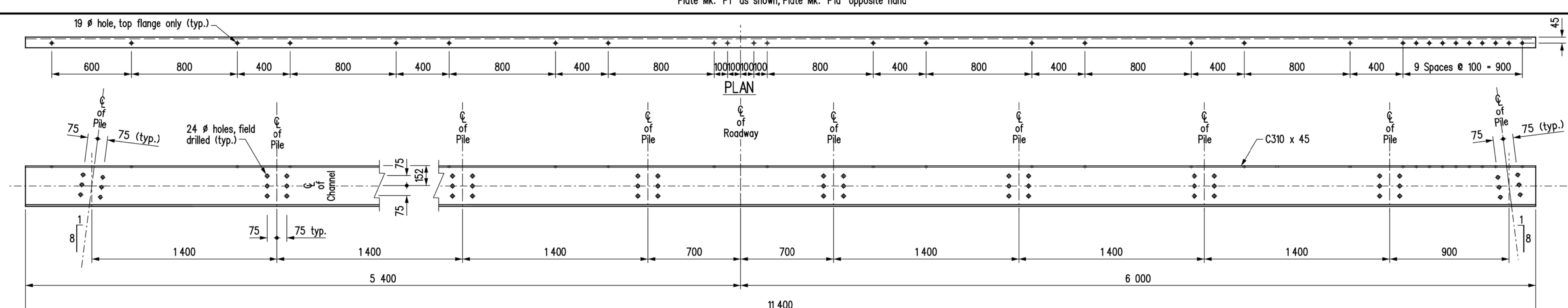
ELEVATION  
STEEL CHANNEL MK "P3"



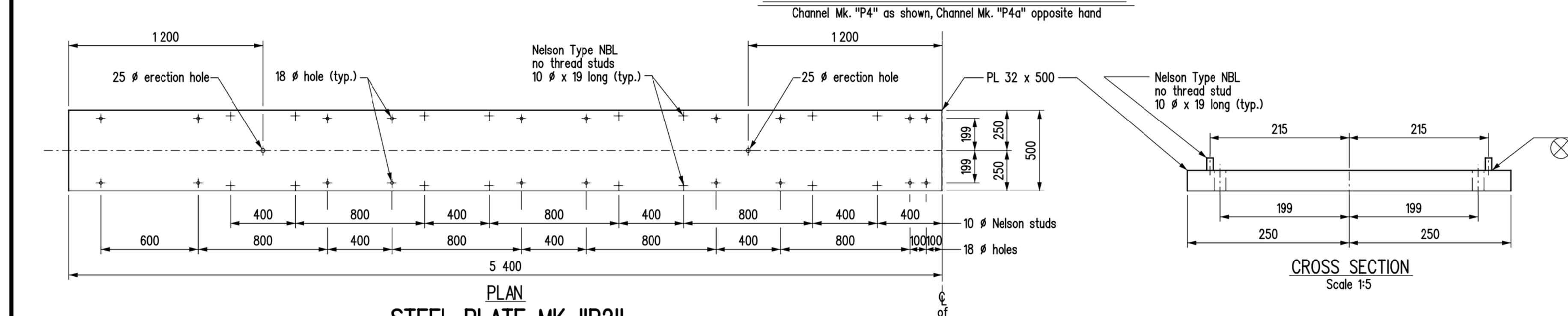
PLAN  
STEEL PLATE MK "P1" & "P1a"  
Plate Mk. "P1" as shown, Plate Mk. "P1a" opposite hand

CROSS SECTION  
Scale 1:5

FOR ABUTMENTS

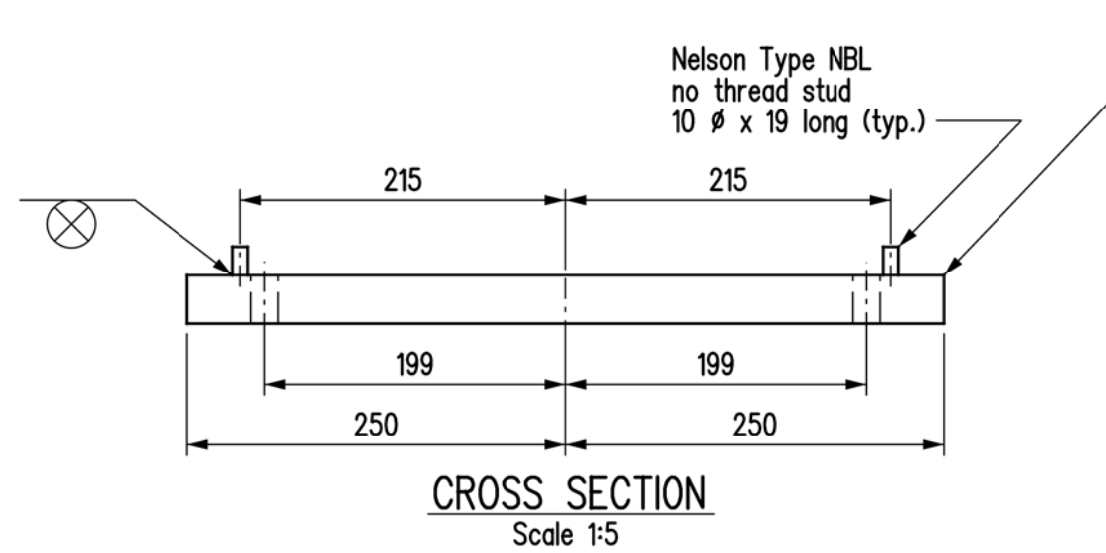


ELEVATION  
STEEL CHANNEL MK "P4" & "P4a"  
Channel Mk. "P4" as shown, Channel Mk. "P4a" opposite hand

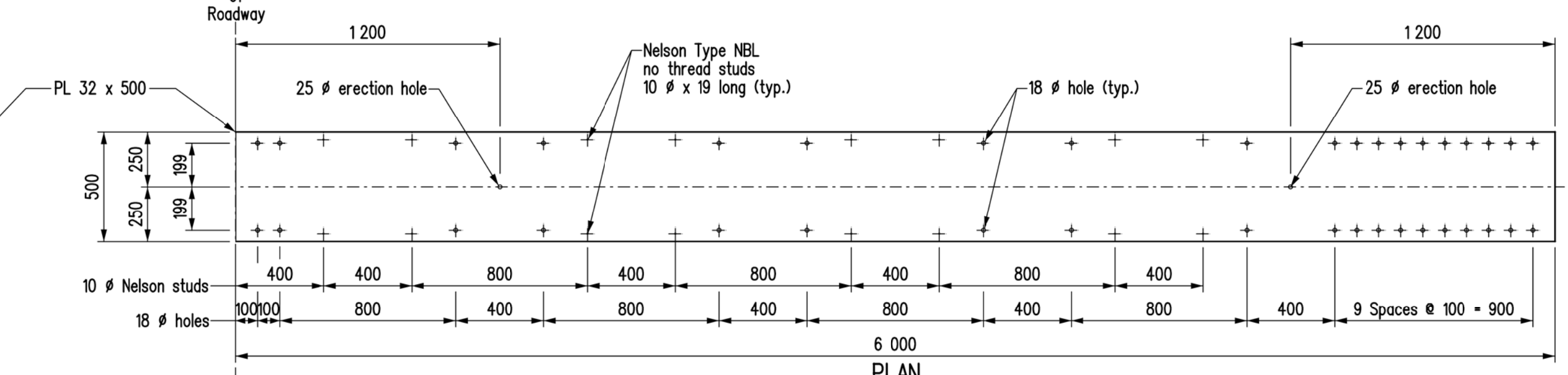


PLAN  
STEEL PLATE MK "P2"

CROSS SECTION  
Scale 1:5



CROSS SECTION  
Scale 1:5

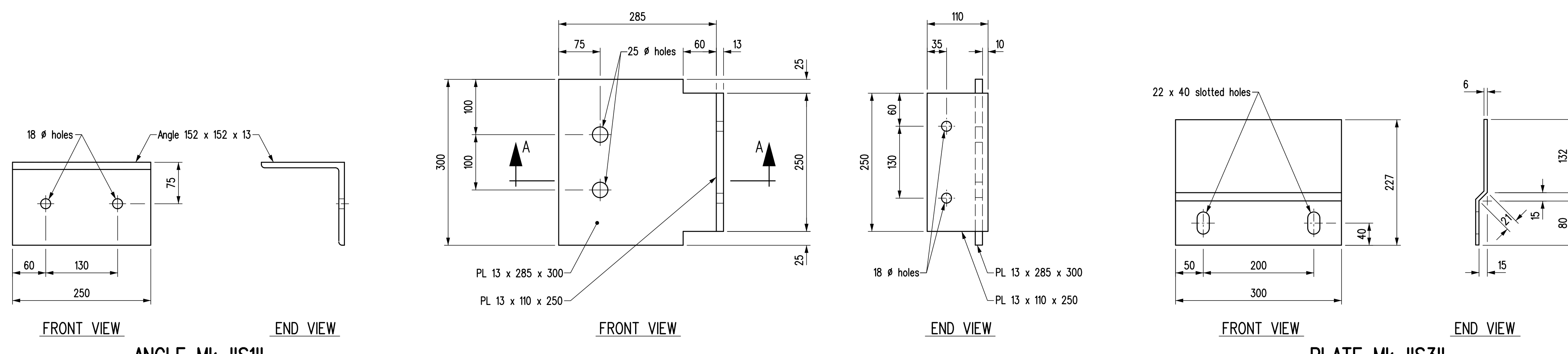


PLAN  
STEEL PLATE MK "P2a"

FOR INTERMEDIATE PILE BENTS

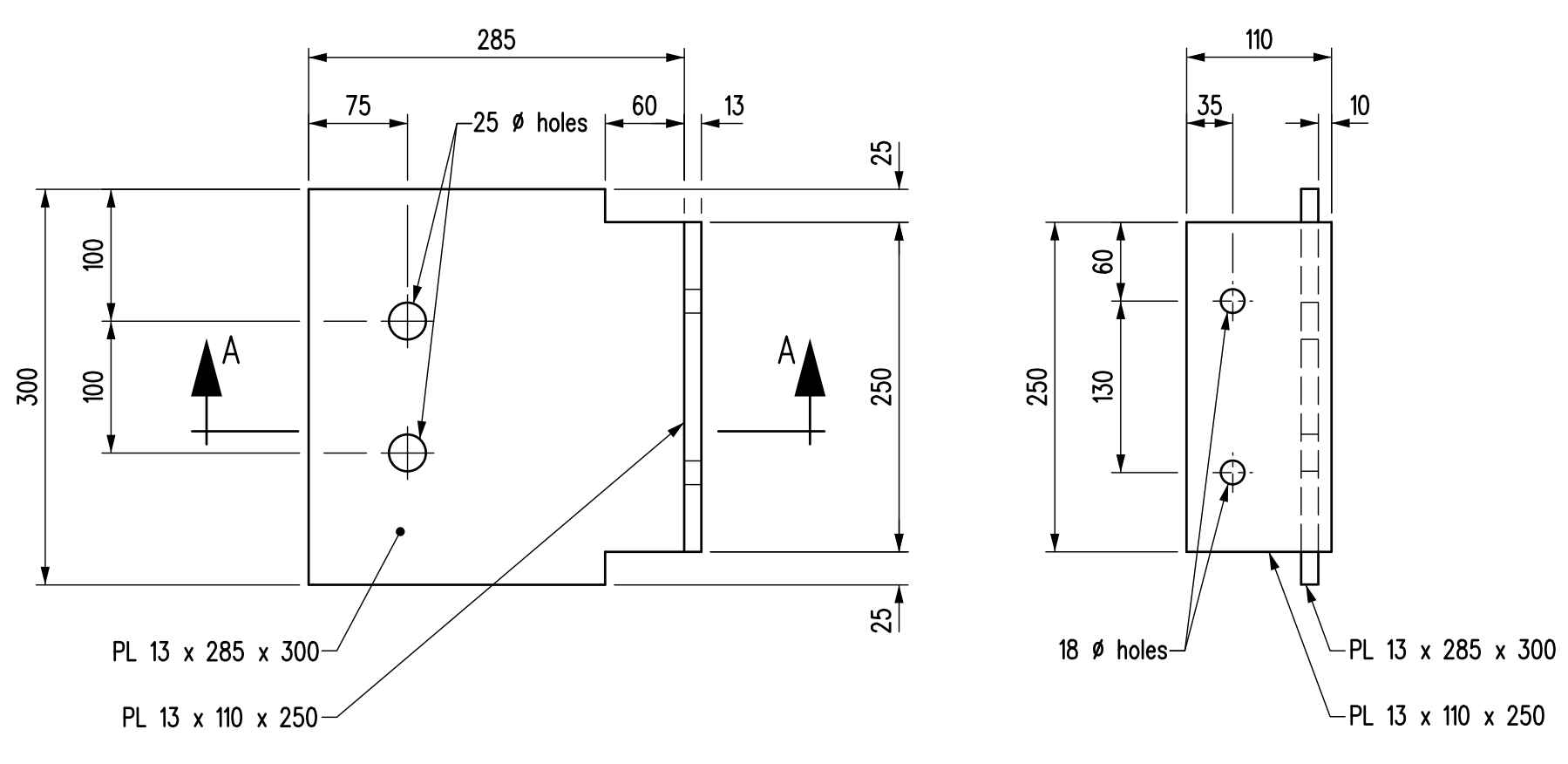
REVISIONS		STEEL PILE CAP DETAILS	
DATE	DESCRIPTION		
		DESIGN SEAL	RECORD SEAL
<p>PLACE ENGINEERS ELECTRONIC SEAL HERE</p>		<p>Manitoba Infrastructure Water Management and Structures</p>	
		<p>RELEASED FOR CONSTRUCTION BY: _____</p>	
DESIGN	BY: _____	EXECUTIVE DIRECTOR OF STRUCTURES	DATE
DETAILS	CHECKED: _____	SCALE: Scale 1:20	SHEET No. 9
	CHECKED: _____	or as shown	SITE No. _____



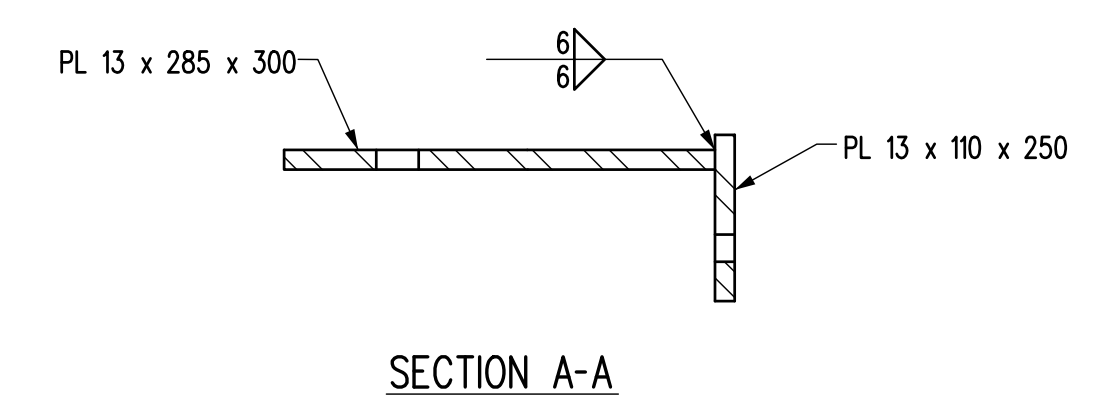


ANGLE Mk. "S1"

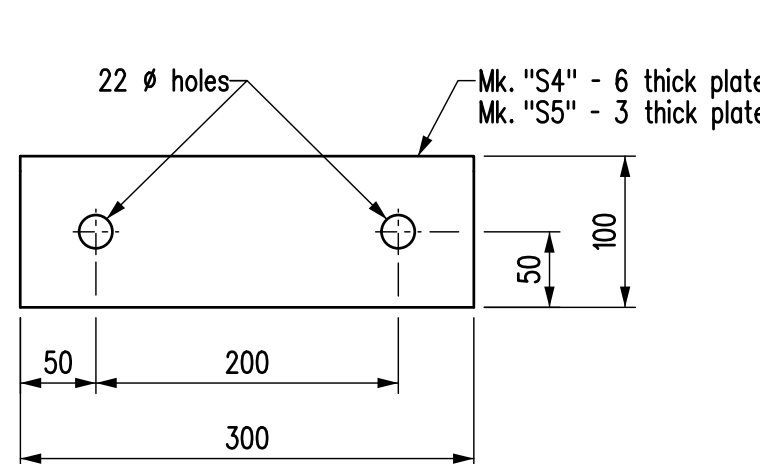
PLATE Mk. "S3"



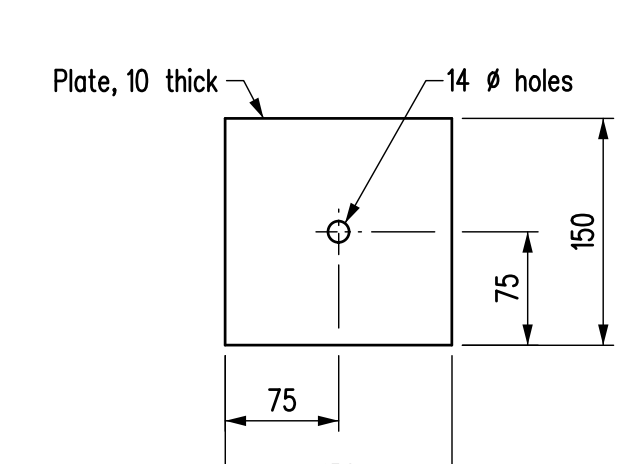
BRACKET Mk. "S2"



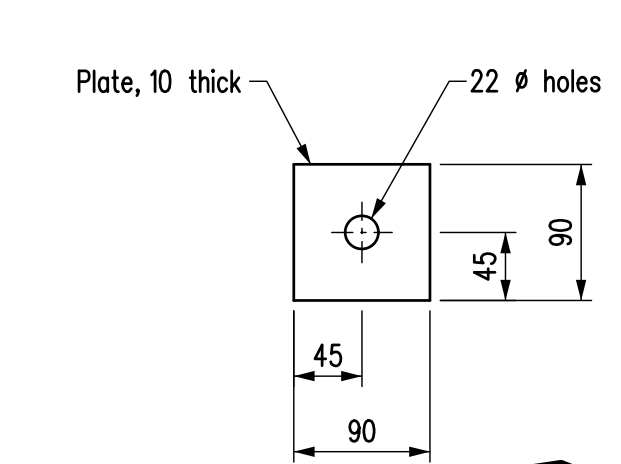
SECTION A-A



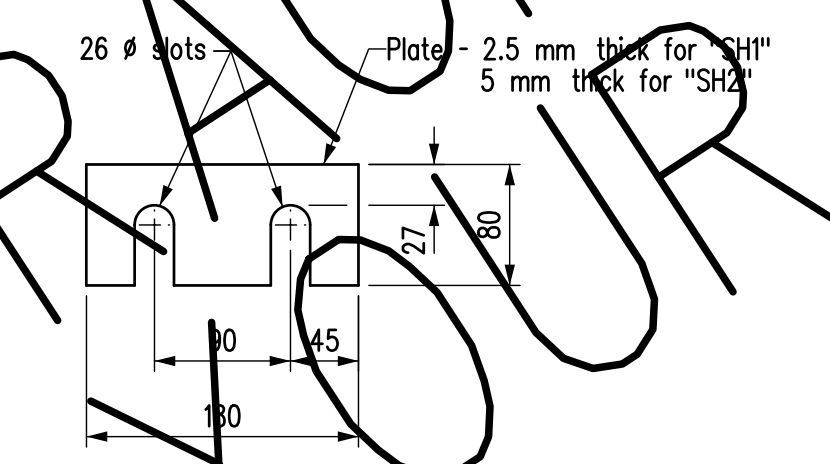
FILLER PLATES Mk. "S4" & "S5"



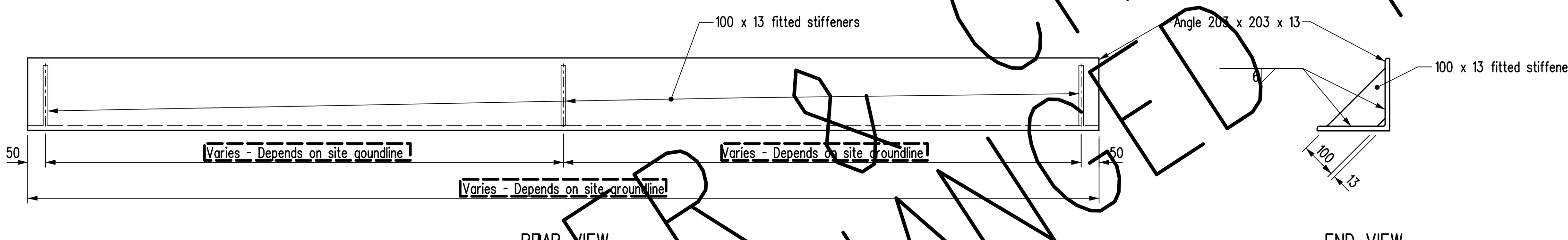
WASHER Mk. "A1"



WASHER Mk. "A2"



SHIM PLATES Mk. "SH1" & "SH2"



CROSS BRACES Mk. "CB1" & "CB2"

REAR VIEW ANGLE Mk. "IU1"

DETAIL "A"

DETAIL "A"

DETAIL "A"

END VIEW

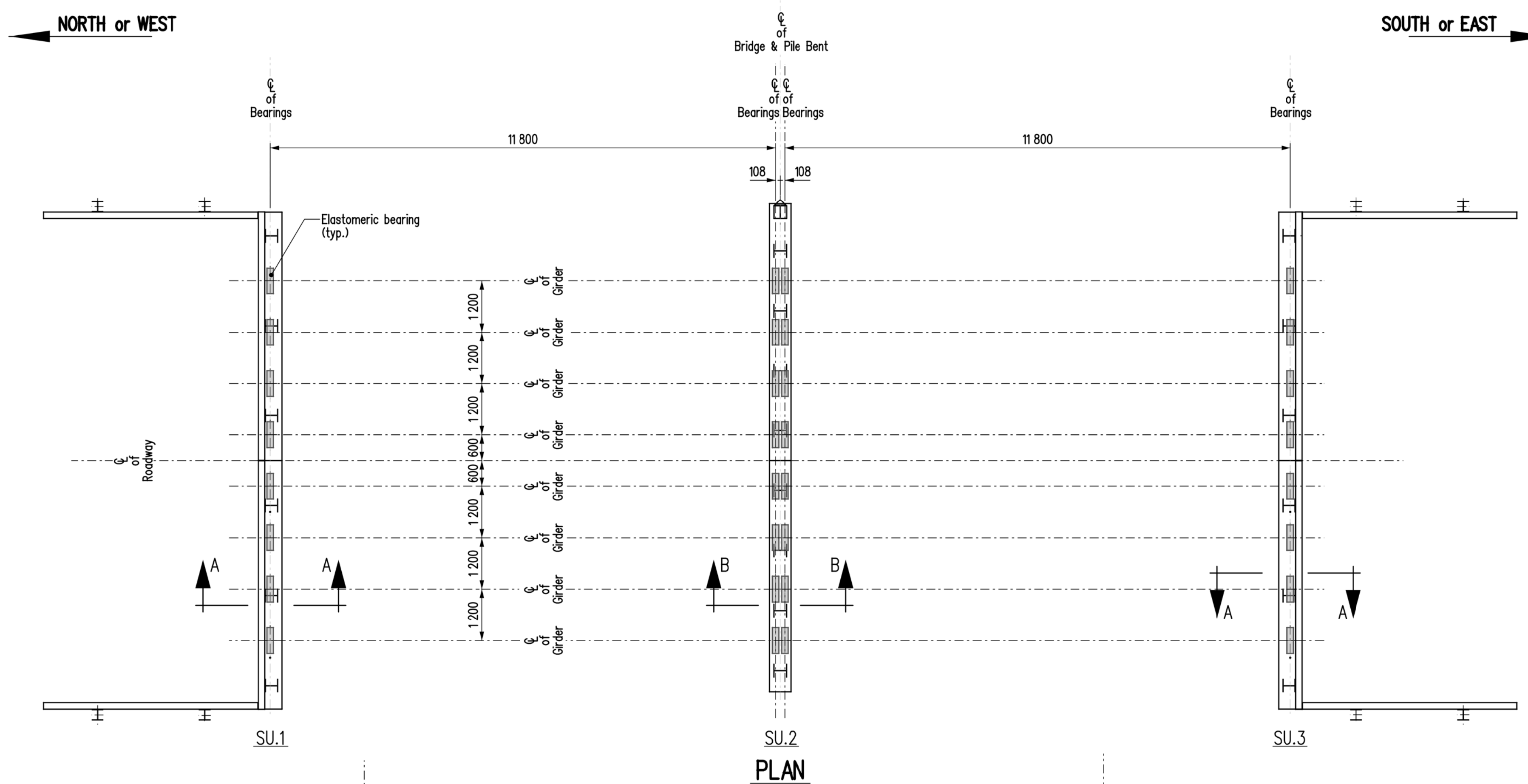
ELEVATION

ICE BREAKER SHOULD BE CHANGED TO CROSS BRACING TO YOUR PRESENTATIONS

BILL OF MISCELLANEOUS METAL 9 600 ROADWAY WIDTH - 2 SPAN - 0 DEGREE SKEW										Site No.
MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS	COMPONENT MASS	MASS PER UNIT	TOTAL MASS	
P1	2	Steel plate	Hot dip galvanized						1602.85	
Each unit to be fabricated from:										
		1 - Steel plate		PL 32x550	5 800	See detail for Abutment	801.328	801.328		
		8 - Nelson Type NBL, no thread studs		10 dia.	19	Part No. 101-063-167	0.012	0.096		
P1a	2	Steel plate	Hot dip galvanized						1602.85	
Each unit to be fabricated from:										
		1 - Steel plate		PL 32x550	5 800	See detail for Abutment	801.328	801.328		
		8 - Nelson Type NBL, no thread studs		10 dia.	19	Part No. 101-063-167	0.012	0.096		
P2	1	Steel plate	Hot dip galvanized						678.43	
Each unit to be fabricated from:										
		1 - Steel plate		PL 32x550	5 400	See detail for Intermediate Bent	678.240	678.240		
		16 - Nelson Type NBL, no thread studs		10 dia.	19	Part No. 101-063-167	0.012	0.192		
P2a	1	Steel plate	Hot dip galvanized						753.77	
Each unit to be fabricated from:										
		1 - Steel plate		PL 32x550	6 000	See detail for Intermediate Bent	753.600	753.600		
		14 - Nelson Type NBL, no thread studs		10 dia.	19	Part No. 101-063-167	0.012	0.168		
R1	4	Steel channel	Hot dip galvanized	C310x45	11 800	See detail for Abutment	518.520	2074.08		
R2	1	Steel channel	Hot dip galvanized	C310x45	11 400	See detail for Intermediate Bent	509.580	509.58		
R4a	1	Steel channel	Hot dip galvanized	C310x45	11 400	See detail for Intermediate Bent	509.580	509.58		
R30	102	A325 bolt assembly	Hot dip galvanized	16 dia.	89	Steel plate to channels	0.245	24.99		
R32	44	A325 bolt assembly	Hot dip galvanized	16 dia.	76	Steel plate to channels C bore holes	0.225	9.90		
R35	252	A325 bolt assembly	Hot dip galvanized	22 dia.	64	Channels to piles	0.461	116.17		
R36	48	A325 bolt assembly	Hot dip galvanized	16 dia.	64	Angles Mk. "S1" to piles & bracket Mk. "S2" to cap	0.205	9.84		
A1	20	Angle	Hot dip galvanized	L 152x152x13	250	As detailed	7.250	145.00		
S2	4	Bracket	Hot dip galvanized			As detailed	11.226	44.90		
S3	16	Plate	Hot dip galvanized	PL 6x300		As detailed	3.223	51.57		
S4	32	Filler plate	Hot dip galvanized	PL 6x100	300	As detailed	1.413	45.22		
S5	16	Filler plate	Hot dip galvanized	PL 3x100	300	As detailed	0.707	11.31		
A1	16	Structural plate washer	Hot dip galvanized	PL 10x150	150	As detailed - One to threaded rod Mk. "TR2"	1.766	28.26		
A2	4	Structural plate washer	Hot dip galvanized	PL 10x90	90	As detailed - One to bolt Mk. "R34"	0.636	5.09		
TR1	32	Threaded rods c/w w/ hex. nuts	Hot dip galvanized	19 dia.	400	Girder to steel cap plate	0.940	30.08		
TR3	32	Threaded rods c/w w/ hex. nuts	Hot dip galvanized	19 dia.	300	Steel plates Mk. "S3" to precast panels	0.660	21.12		
	146	Hardened bevel washer	Hot dip galvanized	for 16 dia. bolts		One to bolts Mk. "R30" & "R32"	0.110	16.06		
	16	Standard flat washer	Hot dip galvanized	for 13 dia. rod		One to threaded rod Mk. "TR2"	0.010	0.16		
	96	Standard flat washer	Hot dip galvanized	for 19 dia. rod		One to "TR1", two to "TR3"	0.020	1.92		
	16	Structural lock washer	Hot dip galvanized	for 12 dia. rod		One to threaded rod Mk. "TR2"	0.010	0.16		
	64	Structural lock washer	Hot dip galvanized	for 19 dia. rod		One to "TR1" & "TR3"	0.020	1.28		
	252	F436 Hardened washer	Hot dip galvanized	for 22 dia. bolts		One to bolt Mk. "R35"	0.032	8.06		
	48	F436 Hardened washer	Hot dip galvanized	for 16 dia. bolts		One to bolt Mk. "R36"	0.014	0.67		
R1	112	A325 bolt assembly	Hot dip galvanized	22 dia.	76	R.C. girder connection	0.499	55.89		
W1	112	Structural flat washer	Hot dip galvanized	for 22 dia. bolts		One to bolt Mk. "R1"	0.050	5.60		
	112	Pair Nord-Lock lock washers		for 22 dia. bolts		One pair to bolt Mk. "R1"	0.020	2.24		
SH1	56	Shim plate	Hot dip galvanized	PL 2.5x80	180	As detailed - use as required	0.231	12.94		
SH2	56	Shim plate	Hot dip galvanized	PL 5x80	180	As detailed - use as required	0.463	25.93		
IP1	2	Plate	Shop Primed	PL277x20	350	See Ice Breaker Details		15.221	30.44	
IP2		Plate	Shop Primed	PL277x20	500	See Ice Breaker Details		21.745	0.00	
IJ1	1	Ice Breaker Unit	Shop Primed						2.35	
Each unit fabricated from:										
		1 - Angle		L203x203x13	0	As detailed	0	0.000		
		- Stiffener Steel Plates		100x13	230	Fitted stiffeners as detailed	2.347	2.347		
CB1	2	Channel	Shop Primed	C200x21	0			0.000	0.00	
CB2	4	Channel	Shop Primed	C200x21	0			0.000	0.00	
<b>TOTAL MASS (kg) = 8438.28</b>										

**NOTES:**  
 1. All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m<sup>2</sup> unless otherwise is stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.  
 2. Seal all welds prior to galvanizing.  
 3. Apply Galvaloy to all field welds and areas where galvanizing has been damaged.  
 4. All bolts and threaded rod in the above Bill shall be Imperial thread.

REVISIONS		STEEL PILE CAP DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
DESIGN SEAL	RECORD SEAL		
PLACE ENGINEERS ELECTRONIC SEAL HERE		Infrastructure Water Management and Structures	
DESIGN BY: _____ CHECKED: _____		SCALE: 1:5 SHEET No. 10	
DETAILS BY: _____ CHECKED: _____		or as shown SITE No. _____	



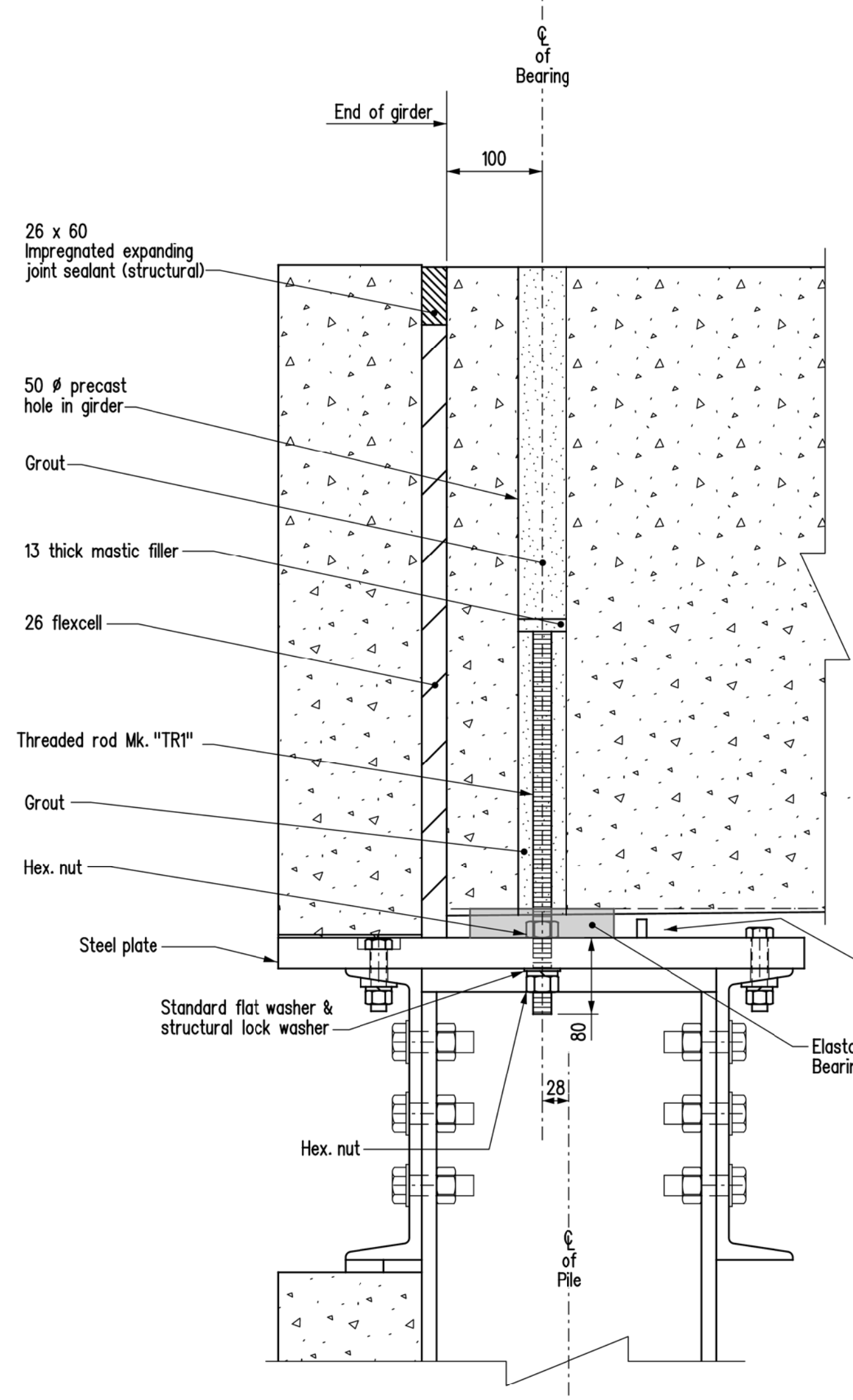
BILL OF BEARINGS			9 600 ROADWAY WIDTH - 2 SPAN	Site No.
No.	LOCATION	DESCRIPTION	REMARKS	
32	SU.1 - SU.3	Elastomeric bearings	As detailed	

**NOTE:**  
 1. Elastomer shall be natural rubber. Elastomer shall be AASHTO low temperature Grade 5 with a minimum shear modulus  $G \geq 0.9$  MPa and a 60 durometer Shore A hardness.  
 2. Internal steel reinforcing plates for laminate bearings shall be rolled mild steel with a minimum yield strength of 300 Mpa.

PLAN  
Scale 1:10

PART CROSS SECTION  
Scale 1:2

**ELASTOMERIC BEARINGS**



**SECTION "A-A"**  
 Threaded rods at SU.1 & SU.3. See sheet No. 6 for layout.  
 Scale 1:5

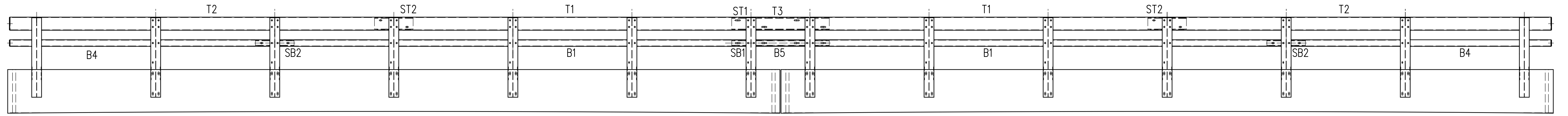


**SECTION "B-B"**  
 Threaded rods at SU.2. See Sheet No. 6 for layout.  
 Scale 1:5

**NOTE:**  
 Threaded rods Mk. "TR1", set exactly on centreline of holes in precast girders and grouted. Field drill 21 dia. hole in steel plate to match girders.

- NOTES:**  
 Re: Girder Erection Operations Behind Abutment Ballast Walls
- Surcharge loading on the backfill resulting from girder erection operations shall be minimized near the precast concrete ballast walls and wingwalls.
  - Where possible, girder erection equipment shall be positioned such that there are no surcharge loads behind the back face of the precast panels within a distance equal to the depth of backfill to the bottom of the panels at the time of girder erection.
  - Should the Contractor propose to encroach on this zone, the following requirements must be satisfied:
    - Submit a girder erection procedure for approval outlining type, configuration, weights and locations of equipment including expected tipping forces on crane outriggers, etc.
    - Perform all precautionary measures outlined by the Department as a result of that submission.
    - All surcharge loads encroaching in this zone must be distributed over an area not less than 2.0 m<sup>2</sup>.

REVISIONS		BEARING AND ERECTION DETAILS	
DATE	BY		
		 Infrastructure Water Management and Structures	
		RELEASED FOR CONSTRUCTION BY: _____ DATE: _____	
		DESIGN BY: _____ CHECKED: _____	EXECUTIVE DIRECTOR OF STRUCTURES _____ DATE: _____ SCALE: 1:75 SHEET No. 11
		DETAILS BY: _____ CHECKED: _____	or as shown SITE No. _____



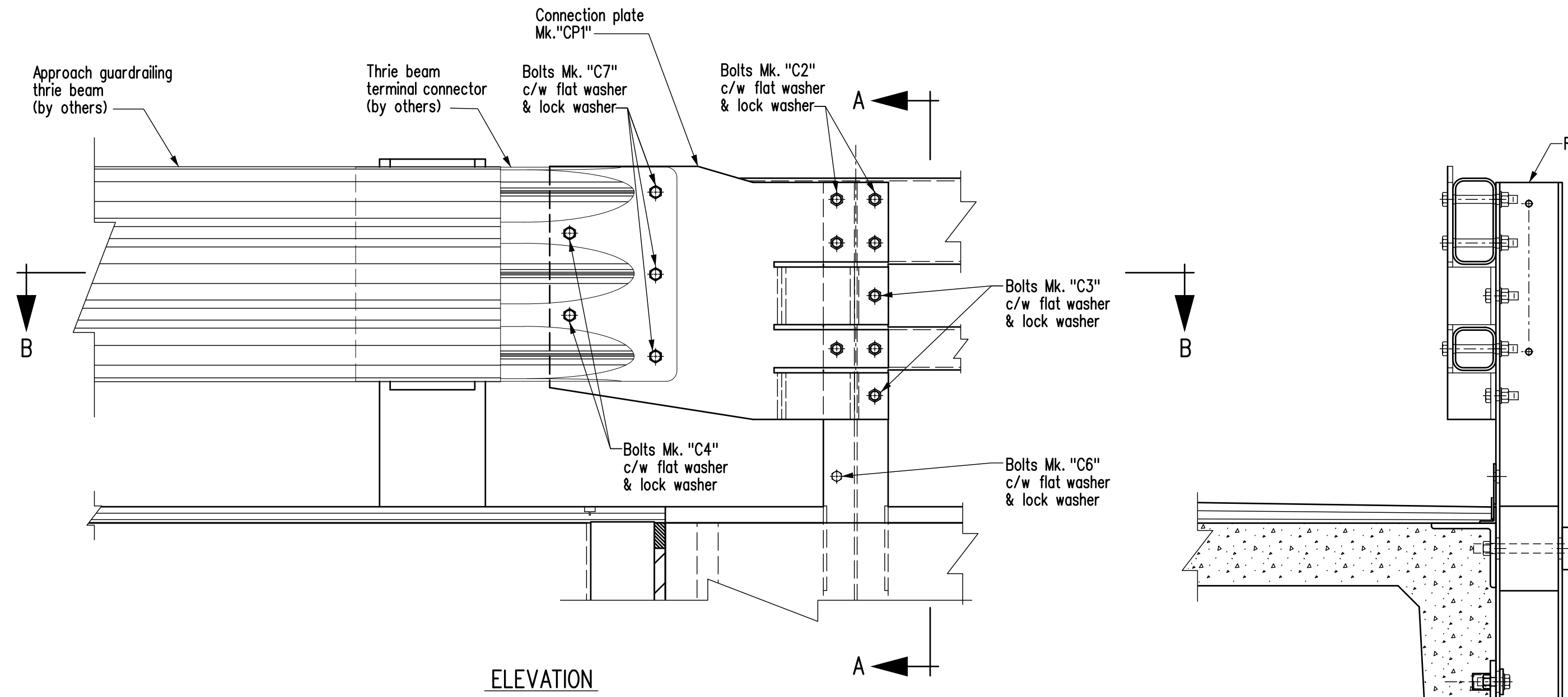
SU.1 END SPAN SU.2 END SPAN SU.3

RAILS				SLEEVES		RAILPOSTS	
T1	T2	B1	B4	ST2	SB2	GP1	GP2
2	2	2	2	2	2	12	2

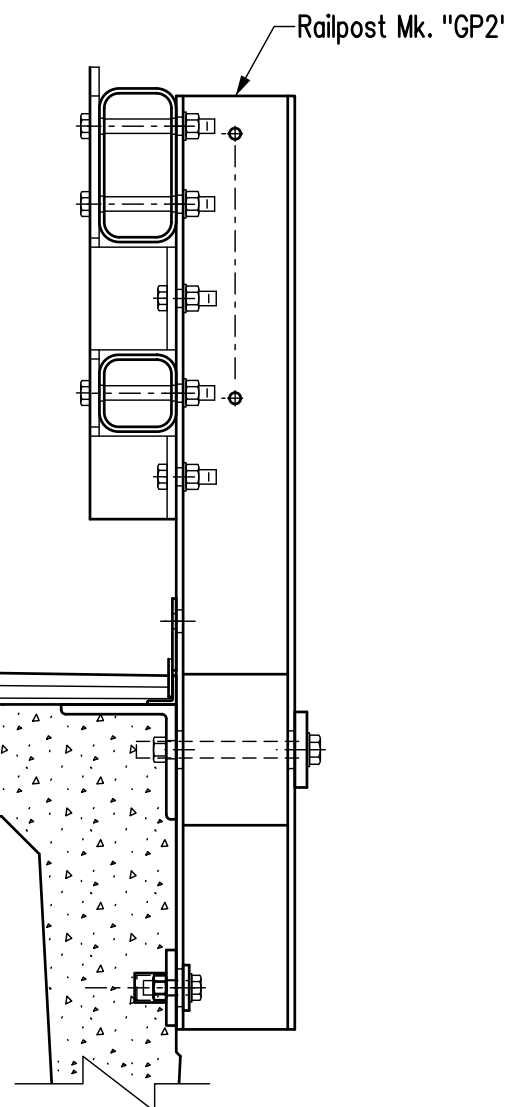
RAILS		SLEEVES	
T3	B5	ST1	SB1
2	2	2	2

RAILS				SLEEVES		RAILPOSTS	
T1	T2	B1	B4	ST2	SB2	GP1	GP2
2	2	2	2	2	2	12	2

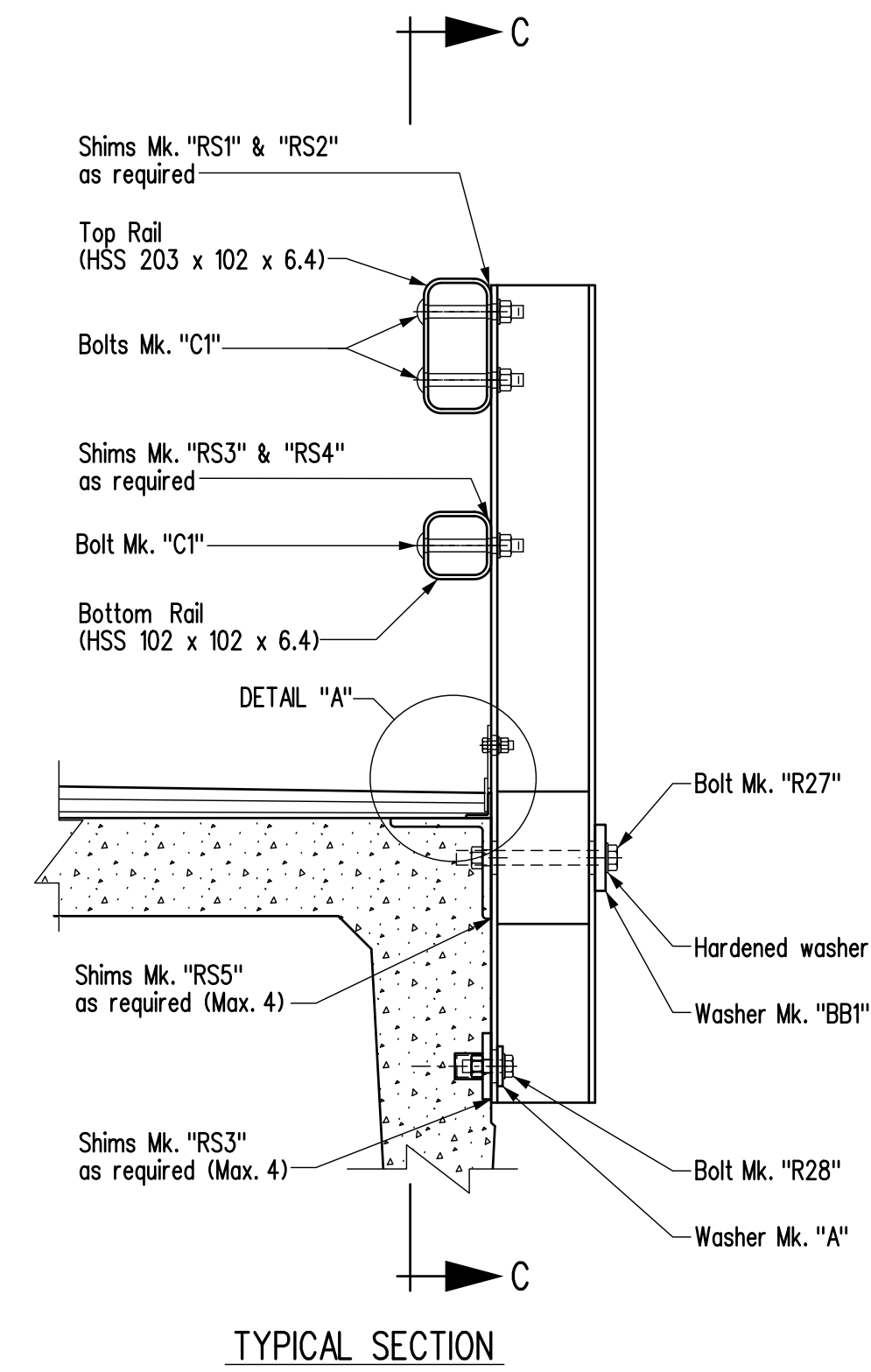
**RAILING LAYOUT**  
Not to Scale



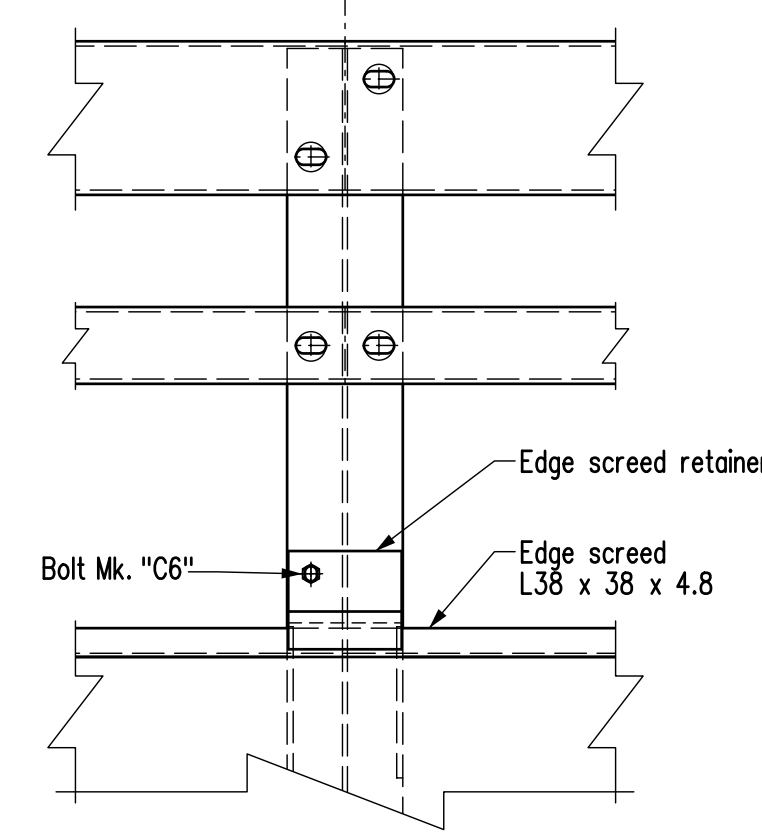
ELEVATION



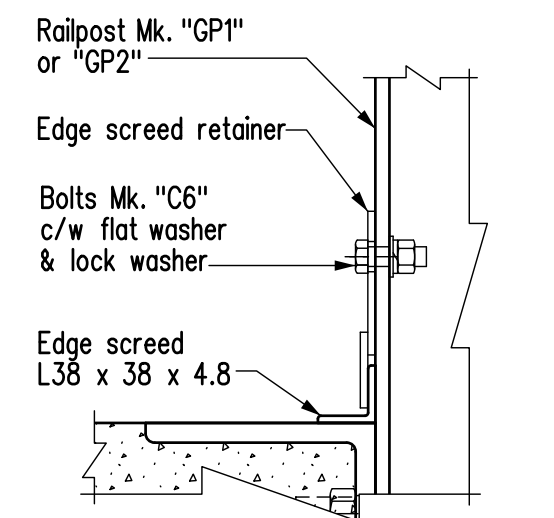
SECTION A-A



TYPICAL SECTION

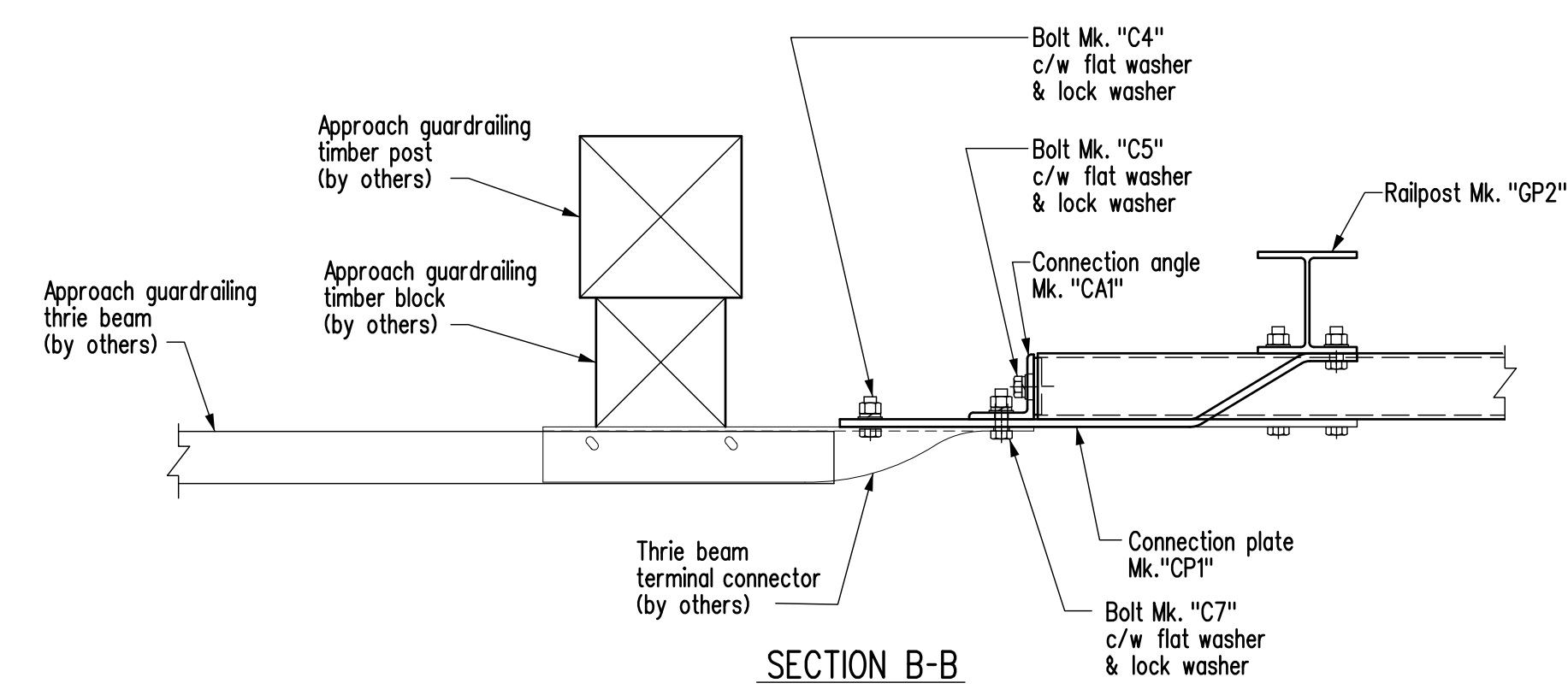


ELEVATION C-C  
Showing edge screed installation detail



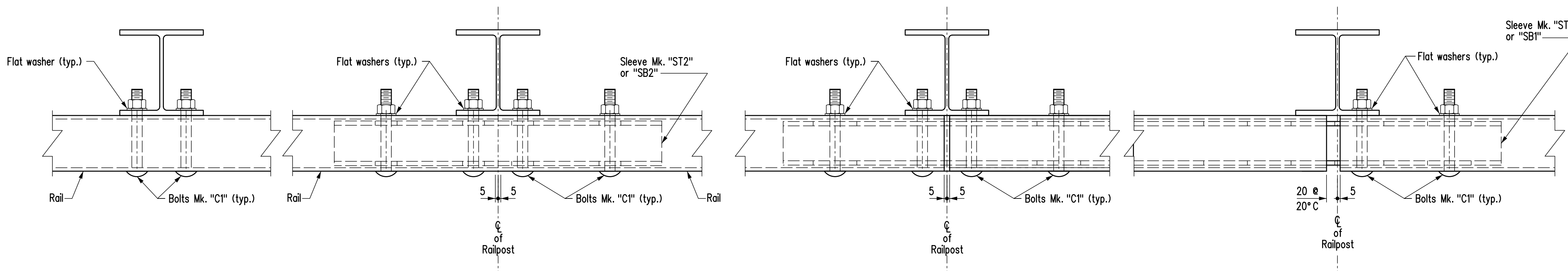
DETAIL "A"  
Showing edge screed installation detail  
Scale 1:5

- NOTES:**
1. All railposts shall be Mk. "GP1" unless noted otherwise.
  2. This sheet to be read in conjunction with Sheets & .



SECTION B-B

**APPROACH RAIL CONNECTION DETAILS**



TYPICAL OF CONTINUOUS RAILS

RAIL END CONNECTION

TYPICAL AT PILE BENT

**RAILING ERECTION DETAILS**

Scale 1:5

**RAILPOST ERECTION DETAILS**

- NOTES:**
1. High strength bolts Mk. "R27" & "R28" shall be tightened by turn-of-nut method as per Specification 1061. These bolts to be supplied by the Girder Fabricator. For quantities see Bill of Miscellaneous Metal on Girder sheet.
  2. High strength bolted connection may be shimmed to a maximum of 12 mm with shims Mk. "RS3" & "RS4".

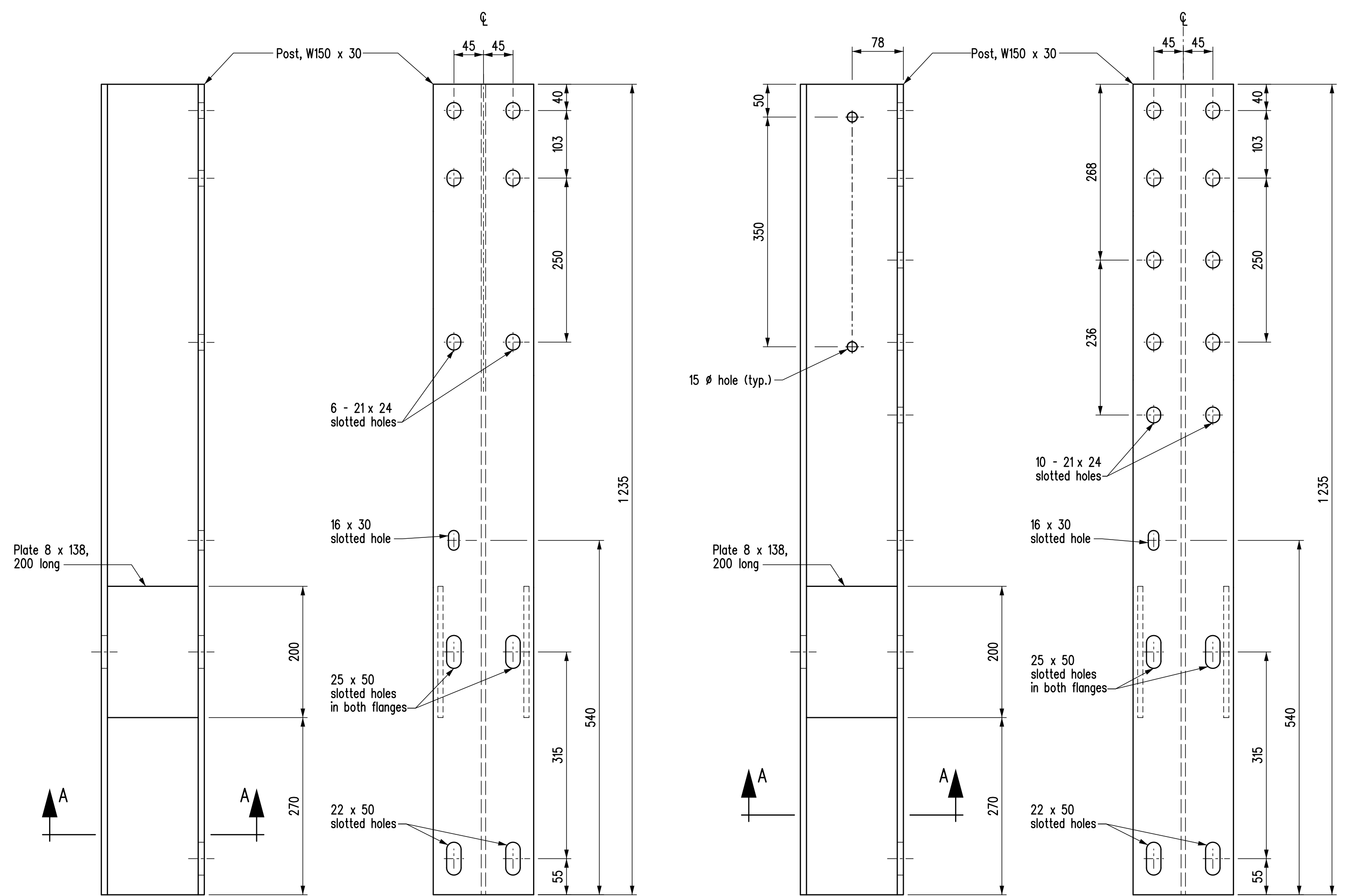
REVISIONS		RAILING LAYOUT AND DETAILS	
DATE	BY	DESIGN	RELEASED FOR CONSTRUCTION BY:
			BY: _____ DATE _____
			EXECUTIVE DIRECTOR OF STRUCTURES
			SCALE: _____ SHEET No. _____
			1:10
			or as shown SITE No. _____



DESIGN  
CHECKED: \_\_\_\_\_  
BY: \_\_\_\_\_  
DETAILS  
CHECKED: \_\_\_\_\_

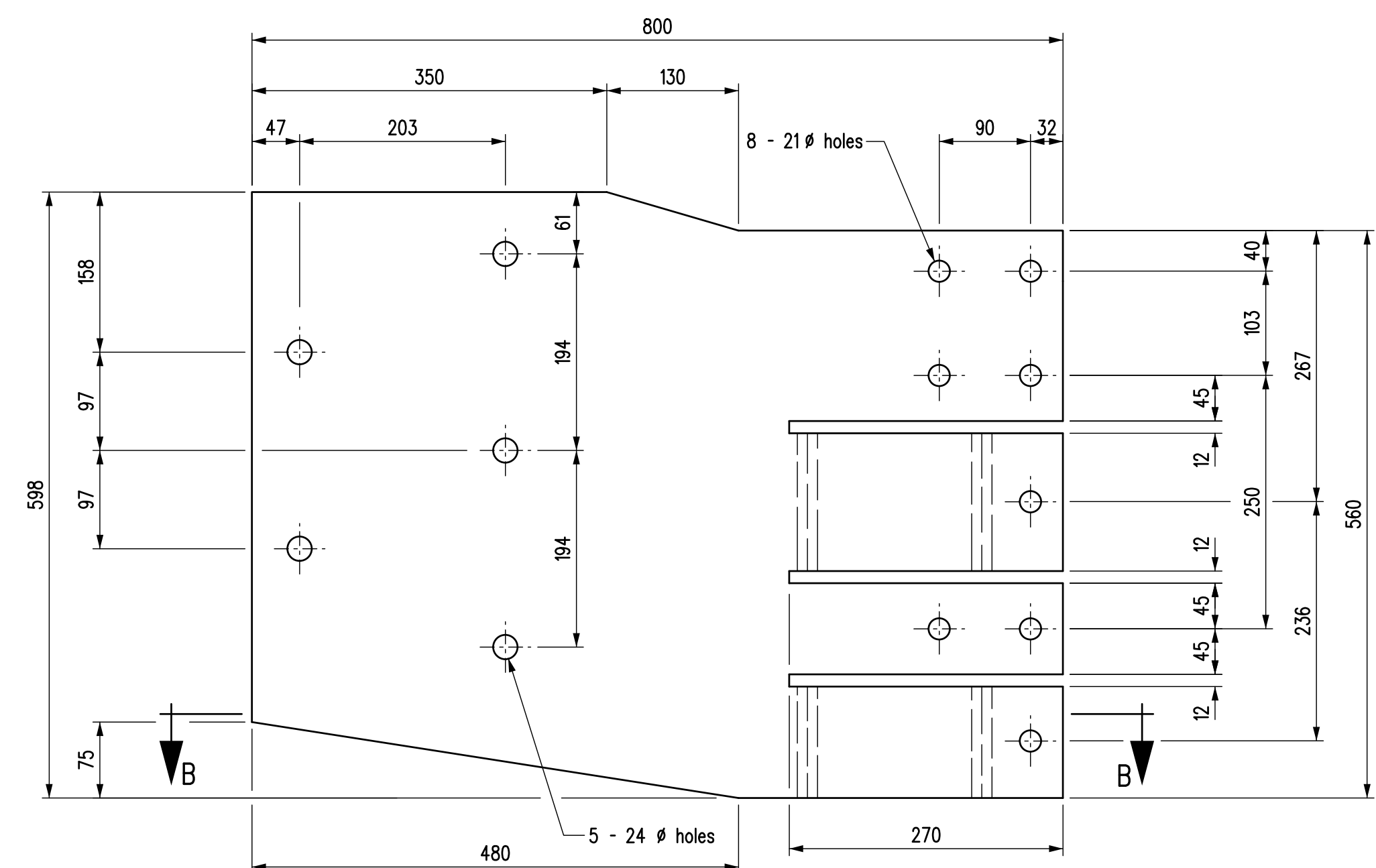
SCALE: \_\_\_\_\_  
BY: \_\_\_\_\_  
CHECKED: \_\_\_\_\_





**RAILPOST MK. "GP1"**

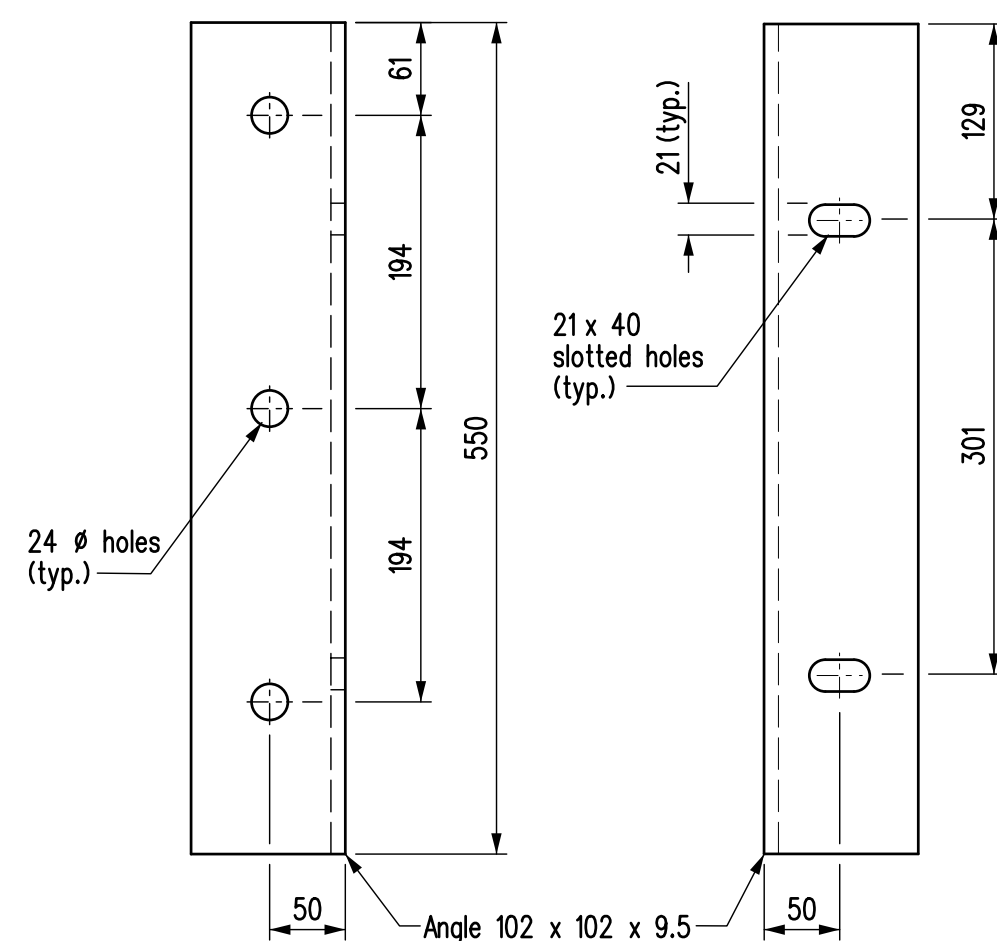
**RAILPOST MK. "GP2"**



**ELEVATION**

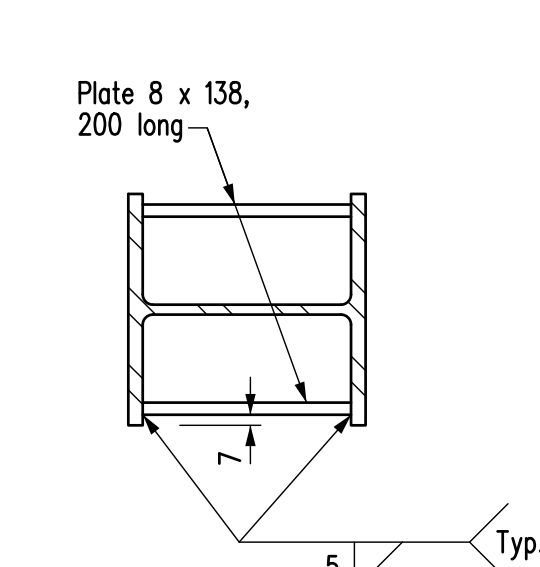
**CONNECTION PLATES MK. "CP1" & "CP2"**

NOTE: Mk. "CP1" shown, Mk. "CP2" opposite hand.



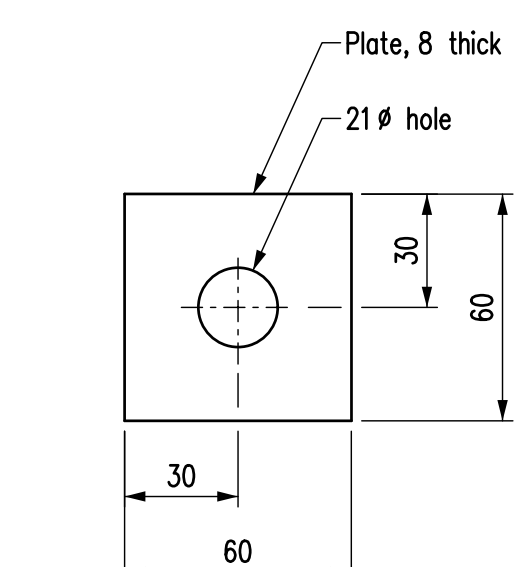
**CONNECTION ANGLES MK. "CA1" & "CA2"**

NOTE: Mk. "CA1" shown, Mk. "CA2" opposite hand.



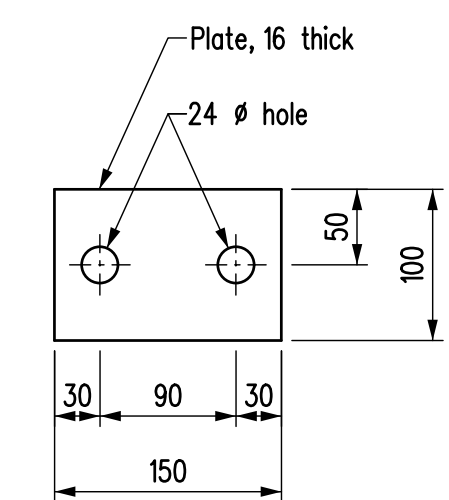
**SECTION A-A**

Typical for railposts Mk. "GP1" & "GP2"

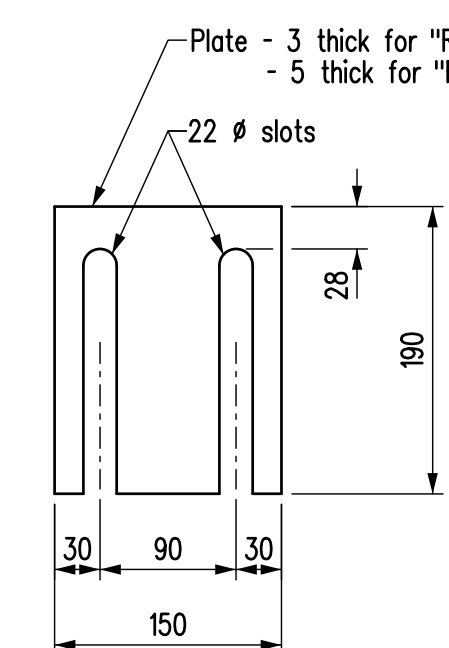


**WASHER MK. "A"**

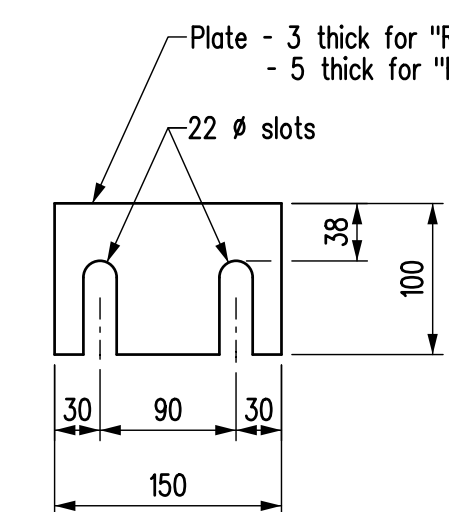
Scale 1:2



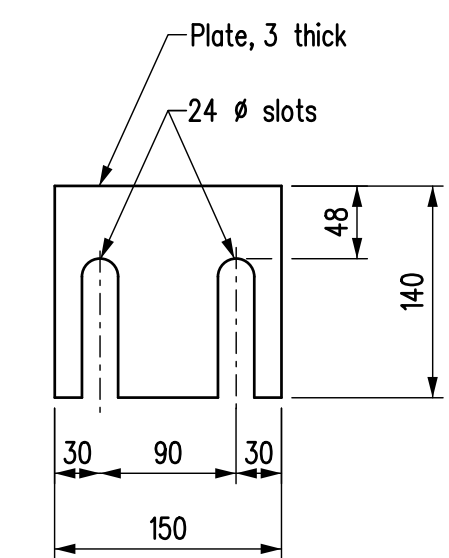
**WASHER MK. "BB1"**



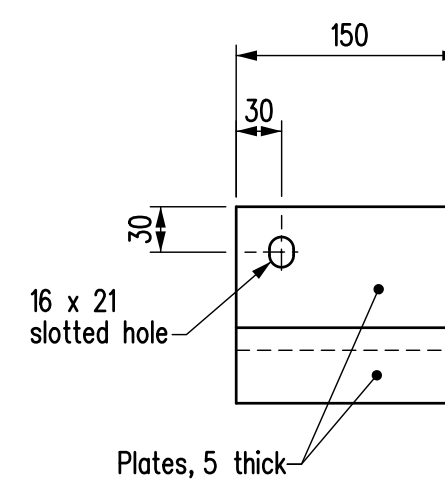
**SHIMS MK. "RS1" & "RS2"**



**SHIMS MK. "RS3" & "RS4"**



**SHIM MK. "RS5"**



**EDGE SCREED RETAINER**

BILL OF MISCELLANEOUS METAL						for BRIDGE RAIL - 2 SPAN			Site No.
MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS	COMPONENT MASS	MASS PER UNIT	TOTAL MASS
GP1	24	Railpost	Hot dip galvanized						953.93
		Each unit to be fabricated from:							
		1 - Post		W150 x 30	1.235	As detailed	36.281	36.281	
		2 - Plates		PL8 x 138	200	As detailed	1.733	3.466	
									39.747
GP2	4	Railpost	Hot dip galvanized						158.51
		Each unit to be fabricated from:							
		1 - Post		W150 x 30	1.235	As detailed	36.161	36.161	
		2 - Plates		PL8 x 138	200	As detailed	1.733	3.466	
									39.827
T1	4	Top rail	Hot dip galvanized						624.75
		Each unit to be fabricated from:							
		1 - Hollow structural section		HSS203x102x6.4	5.540	As detailed		156.188	
									677.98
T2	4	Top rail	Hot dip galvanized						167.982
		Each unit to be fabricated from:							
		1 - Hollow structural section		HSS203x102x6.4	5.962	As detailed	167.982	167.982	
		1 - Plate		PL12 x 88	190	As detailed	1.514	1.514	
									169.496
T3	2	Top rail	Hot dip galvanized						50.13
		Each unit to be fabricated from:							
		1 - Hollow structural section		HSS203x102x6.4	891	As detailed		25.066	
B1	4	Bottom rail	Hot dip galvanized						535.02
		Each unit to be fabricated from:							
		1 - Hollow structural section		HSS102x102x6.4	7.390	As detailed		133.755	
B4	4	Bottom rail	Hot dip galvanized						300.24
		Each unit to be fabricated from:							
		1 - Hollow structural section		HSS102x102x6.4	4.112	As detailed		74.392	
		1 - Plate		PL12 x 88	88	As detailed		0.668	
									75.060
B5	2	Bottom rail	Hot dip galvanized						32.13
		Each unit to be fabricated from:							
		1 - Hollow structural section		HSS102x102x6.4	891	As detailed		16.067	
ST1	2	Sleeve	Hot dip galvanized						83.00
		Each unit to be fabricated from:							
		2 - Plates			1.516	As detailed	20.749	41.498	
ST2	4	Sleeve	Hot dip galvanized						66.10
		Each unit to be fabricated from:							
		2 - Plates			600	As detailed	8.263	16.526	
SB1	2	Sleeve	Hot dip galvanized						45.49
		Each unit to be fabricated from:							
		2 - Plates			1.516	As detailed	11.372	22.744	
SB2	4	Sleeve	Hot dip galvanized						37.17
		Each unit to be fabricated from:							
		2 - Plates			600	As detailed	4.646	9.292	
CP1	2	Connection plate	Hot dip galvanized						41.605
CP2	2	Connection plate	Hot dip galvanized						41.605
CA1	2	Connection angle	Hot dip galvanized	L102x102x9.5	550	As detailed	7.864	15.73	
CA2	2	Connection angle	Hot dip galvanized	L102x102x9.5	550	As detailed	7.864	15.73	
A	56	Washer	Hot dip galvanized	PL8x60	60	As detailed	0.226	12.66	
BB1	28	Washer	Hot dip galvanized	PL16x100	150	As detailed	1.884	52.75	
C1	120	Bolts c/w hex nuts	Hot dip galvanized	19 dia.	150	Round head, square neck bolt c/w 1 hex nut	0.424	50.88	
C2	24	Bolts c/w hex nuts	Hot dip galvanized	19 dia.	165	Hex bolt c/w 1 hex nut	0.466	11.18	
C3	8	Bolts c/w hex nuts	Hot dip galvanized	19 dia.	65	Hex bolt c/w 1 hex nut	0.249	1.99	
C4	8	Bolts c/w hex nuts	Hot dip galvanized	22 dia.	50	Hex bolt c/w 1 hex nut	0.327	2.62	
C5	8	Bolts - no nuts	Hot dip galvanized	19 dia.	38	Hex bolt - no nuts	0.145	1.16	
C6	24	Bolts c/w hex nuts	Hot dip galvanized	13 dia.	38	Hex bolt c/w 1 hex nut	0.070	1.68	
C7	12	Bolts c/w hex nuts	Hot dip galvanized	22 dia.	65	Hex bolt c/w 1 hex nut	0.215	2.58	
8	Edge screed angle	Hot dip galvanized		L38x38x4.8	6.000	As detailed		128.16	
24	Edge screed retainers	Hot dip galvanized						20.23	
	Each unit to be fabricated from:								
		1 - Plate		PL5x95	150	As detailed	0.549	0.549	
		1 - Plate		PL5x50	150	As detailed	0.294	0.294	
									0.843
20	Standard flat washer	Hot dip galvanized		for 22 dia. bolts		1 per bolt Mk. "C4" & "C7"	0.032	0.64	
160	Standard flat washer	Hot dip galvanized		for 19 dia. bolts		1 per bolt Mk. "C1", "C2", "C3" & "C5"	0.022	3.52	
24	Standard flat washer	Hot dip galvanized		for 13 dia. bolts		1 per bolt Mk. "C6"	0.010	0.24	
160	Standard lock washer	Hot dip galvanized		for 19 dia. bolts		1 per bolt Mk. "C1", "C2", "C3" & "C5"	0.019	3.04	
24	Standard lock washer	Hot dip galvanized		for 13 dia. bolts		1 per bolt Mk. "C6"	0.007	0.17	
20	Standard lock washer	Hot dip galvanized		for 22 dia. bolts		1 per bolt Mk. "C4" & "C7"	0.027	0.54	
RS1	48	Shims	Hot dip galvanized	PL3x150	190	As detailed	0.506	24.29	
RS2	48	Shims	Hot dip galvanized	PL5x150	190	As detailed	0.843	40.46	
RS3	144	Shims	Hot dip galvanized	PL3x150	100	As detailed	0.291	41.90	
RS4	48	Shims	Hot dip galvanized	PL5x150	100	As detailed	0.486	23.33	
RS5	96	Shims	Hot dip galvanized	PL3x140	150	As detailed	0.394	37.82	
									<b>TOTAL MASS (kg) = 4224.18</b>

**NOTES:**

- HSS rail shall conform to CAN/CSA-G40.21-M92 Grade 350W.
- All steel plates shall conform to the requirements of CAN/CSA-G40.21-M92 Grade 300W.
- W150 x 30 railpost shall conform to CAN/CSA-G40.21-M92 Grade 350W.
- Welding shall meet the current requirements of the American Welding Society, Structural Welding Code ANSI/AASHTO/AWS D1.5.
- All bolts shall conform to the requirements of ASTM A307 or approved equal, unless noted otherwise.
- All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m<sup>2</sup> unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.
- Seal all welds prior to galvanizing.
- Apply Galvalloy to all field welds and areas where galvanizing has been damaged.
- All bolts and threaded rod in the above Bill shall be Imperial thread.

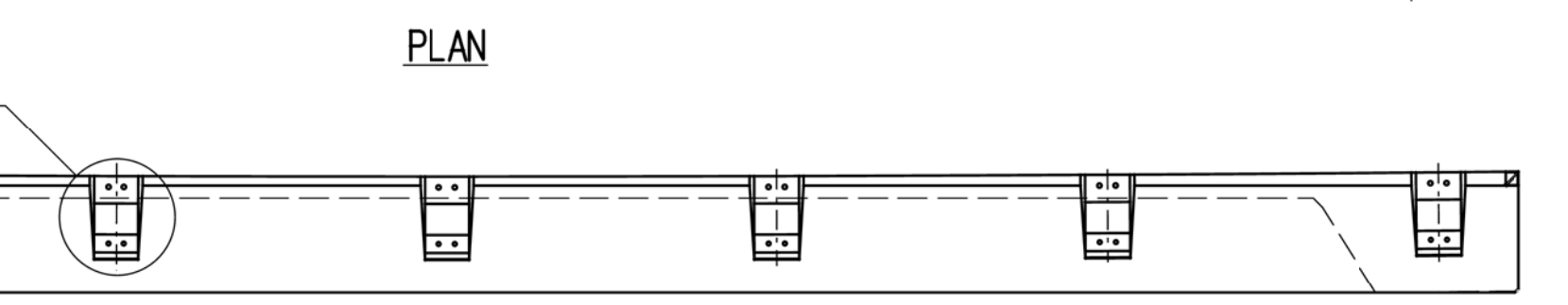
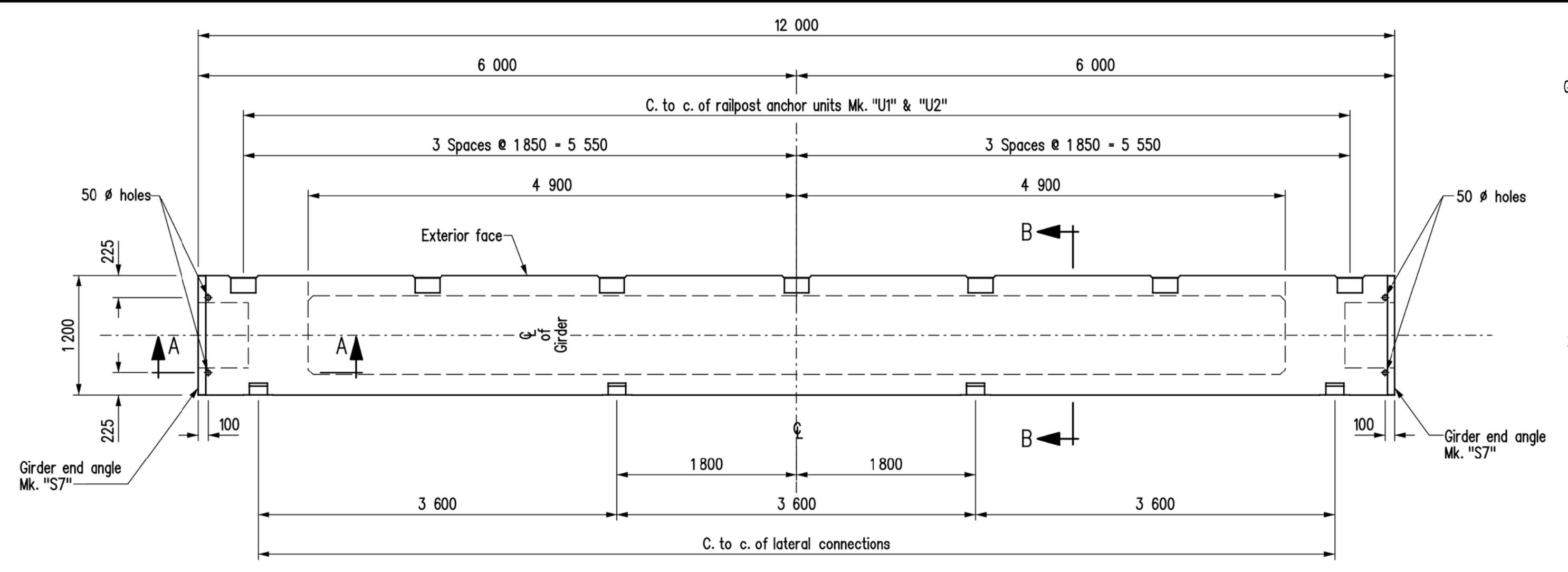
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DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:

Water Management and Structures

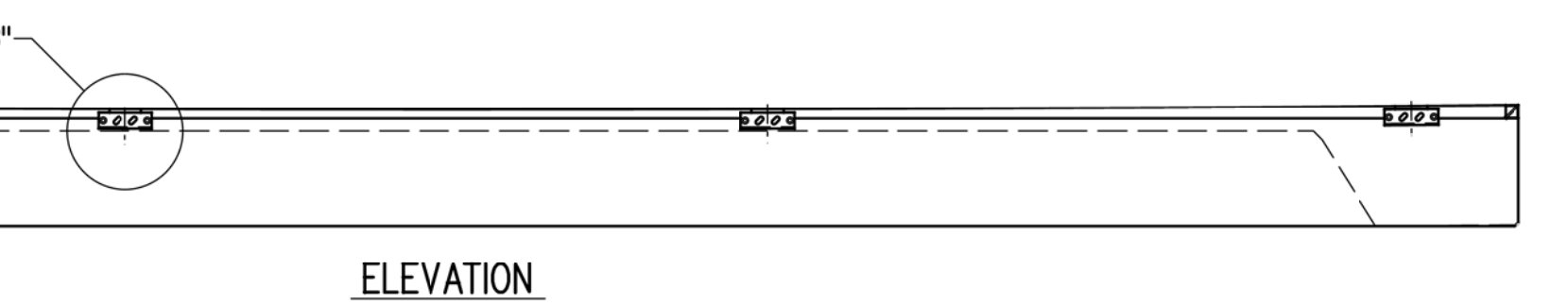
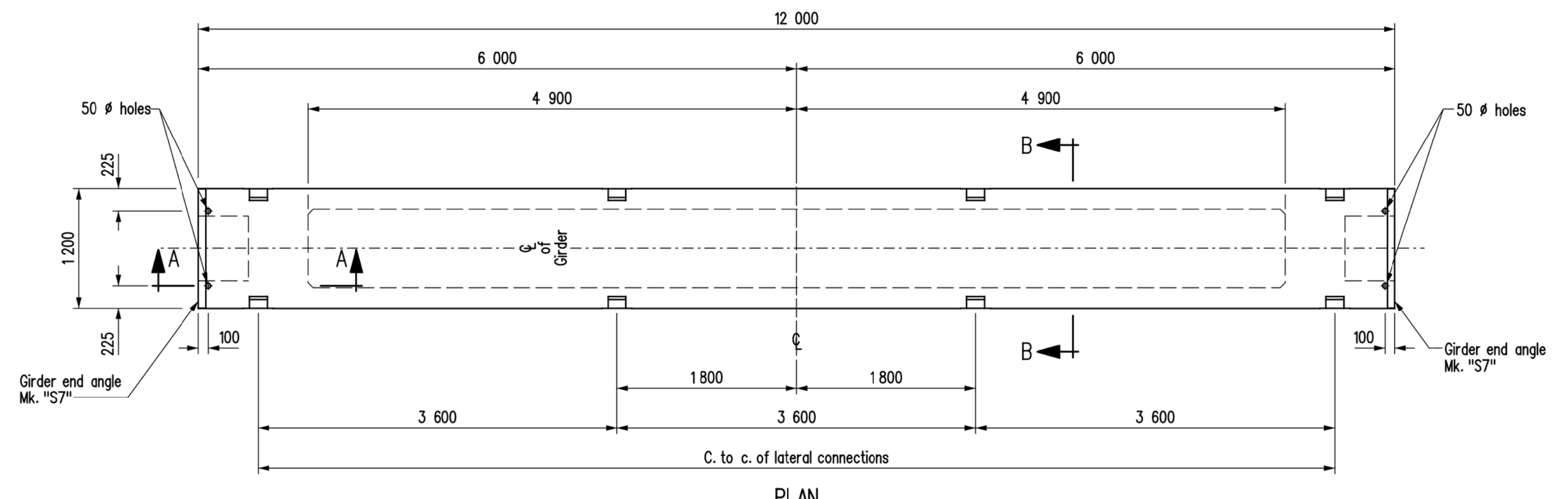
EXECUTIVE DIRECTOR OF STRUCTURES

DATE

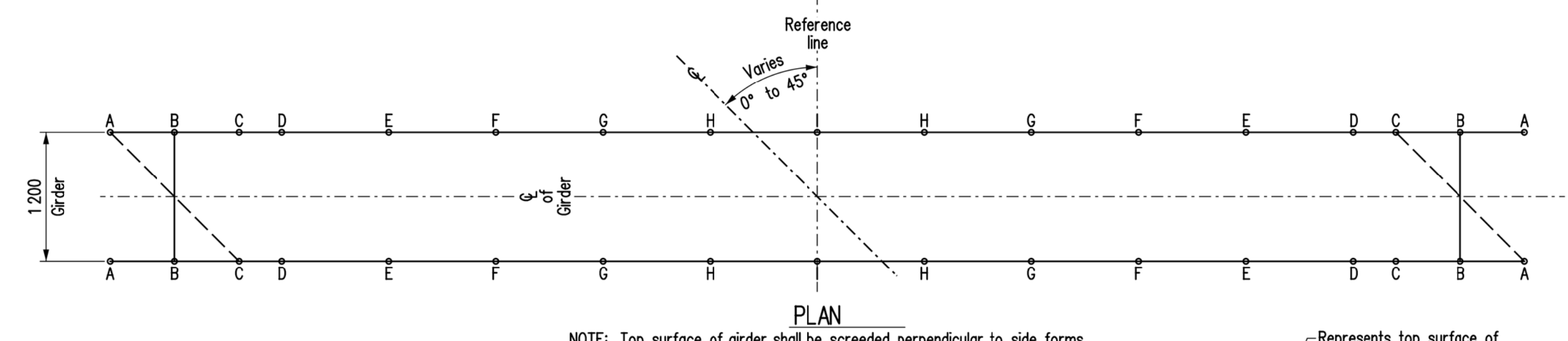
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	CHECKED: _____		
DETAILS	BY: _____	or as shown	SITE No. _____
	CHECKED: _____		



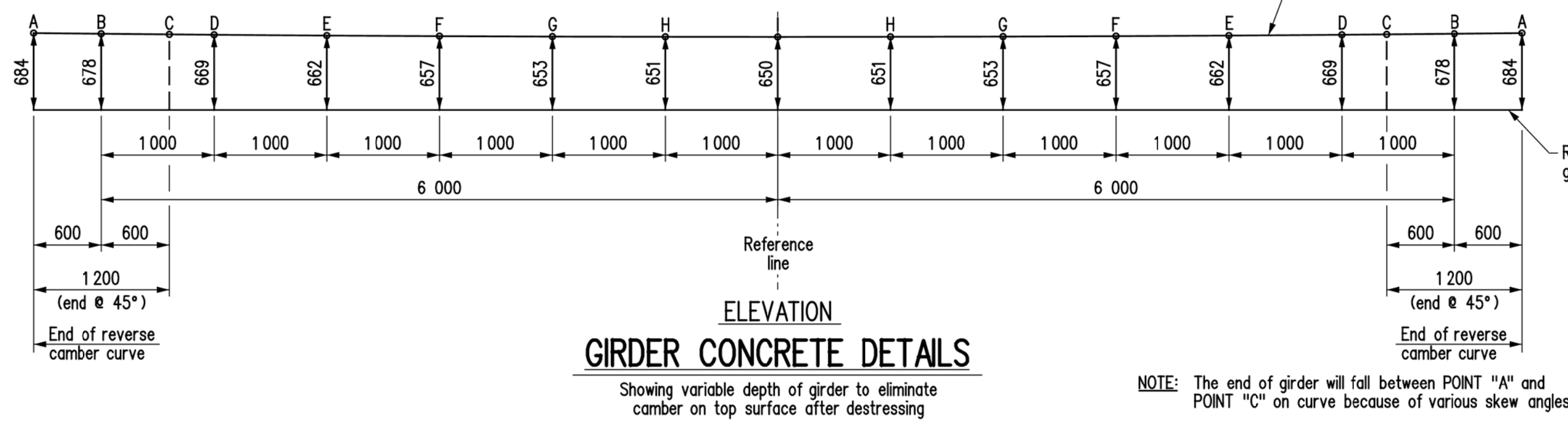
EXTERIOR ELEVATION  
EXTERIOR GIRDER MK. "G1"



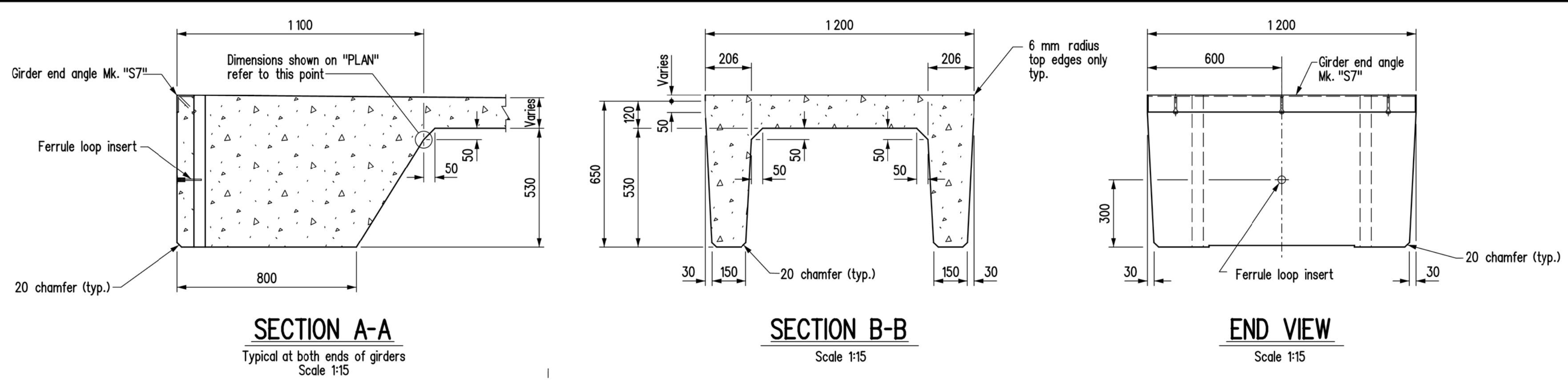
ELEVATION  
INTERIOR GIRDER MK. "G2"



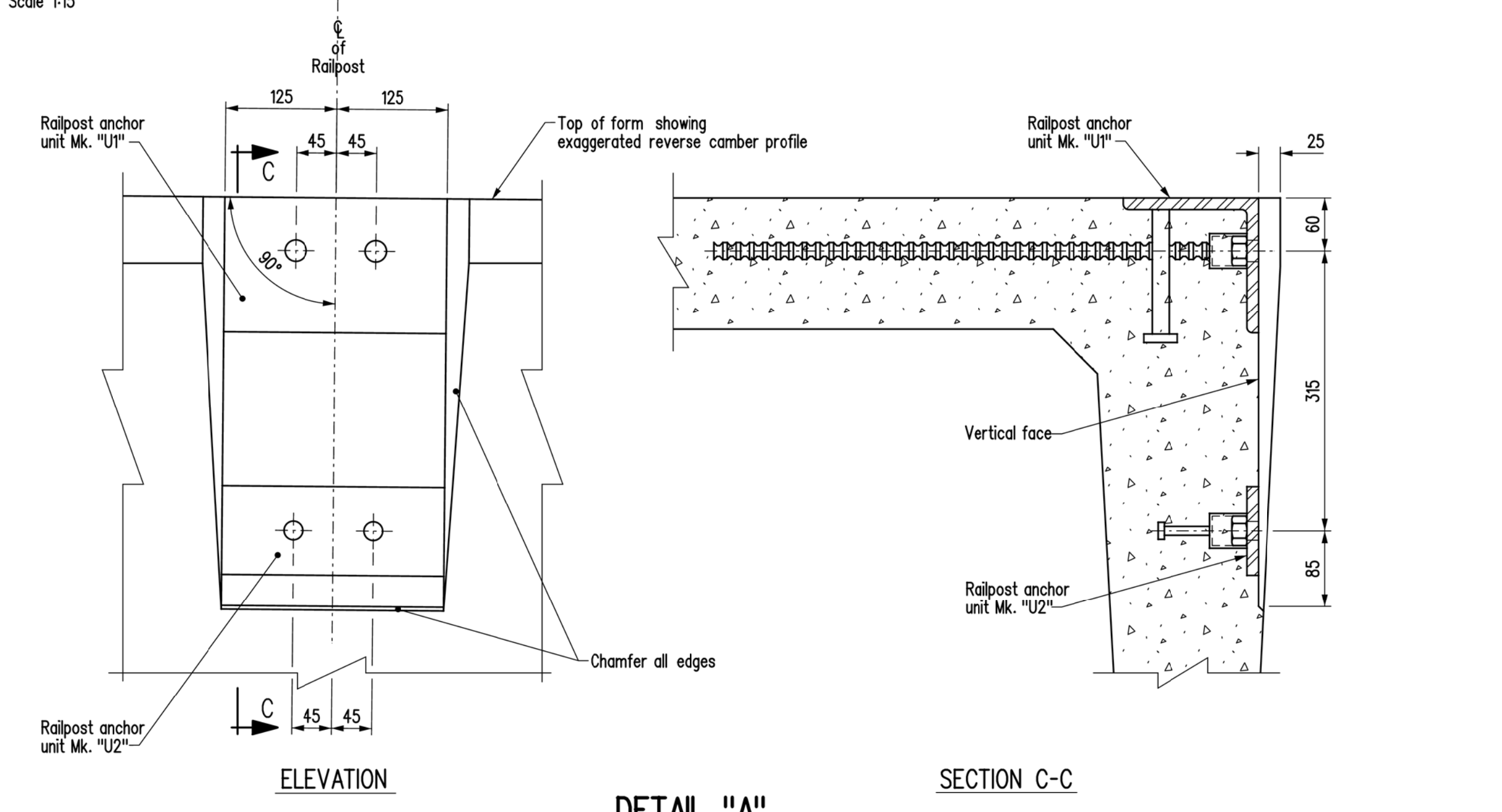
GIRDER CONCRETE DETAILS



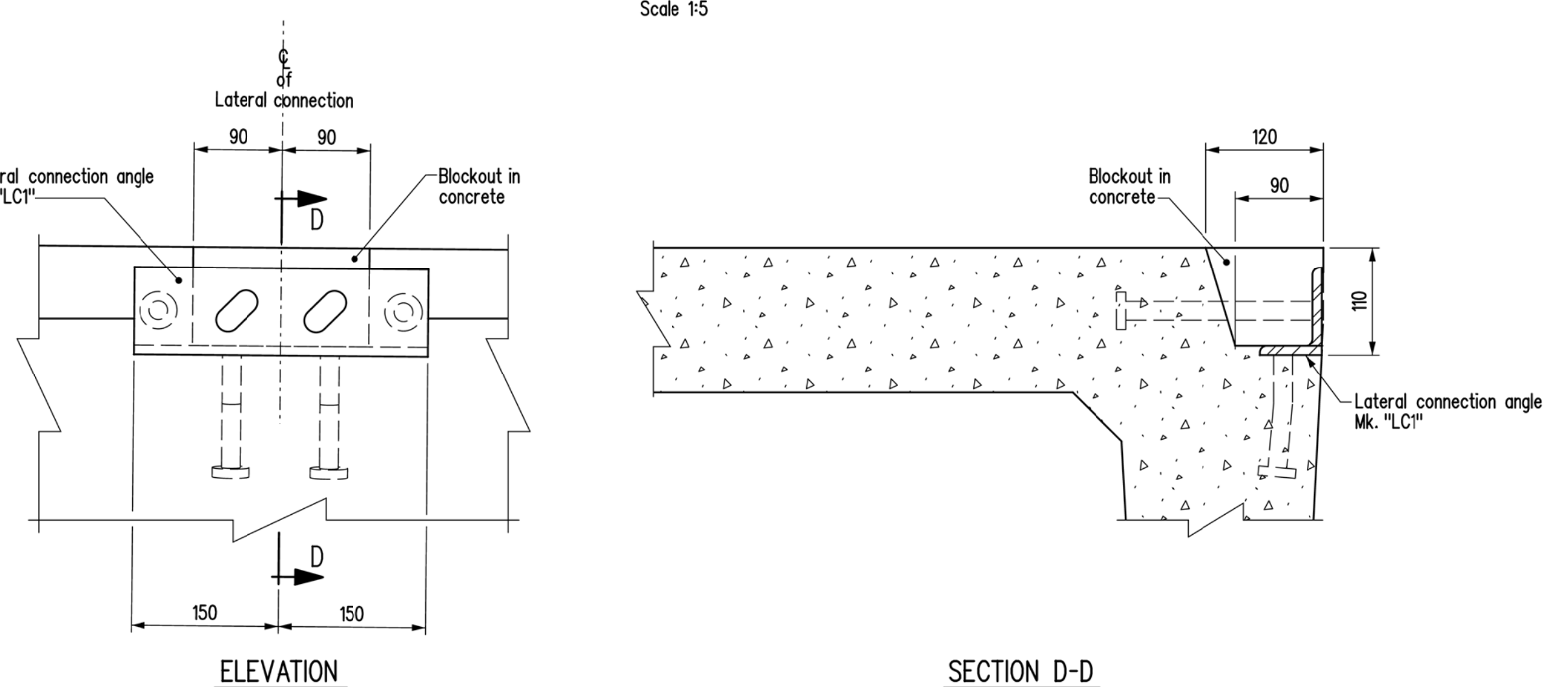
ELEVATION  
GIRDER CONCRETE DETAILS



SECTION A-A Typical at both ends of girders Scale 1:15  
SECTION B-B Scale 1:15  
END VIEW Scale 1:15



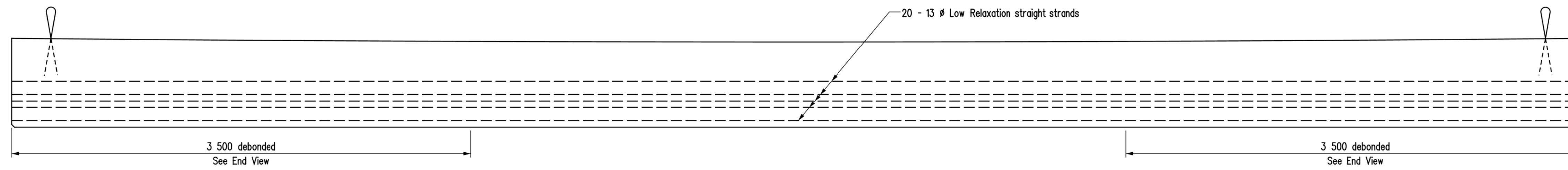
ELEVATION  
SECTION C-C



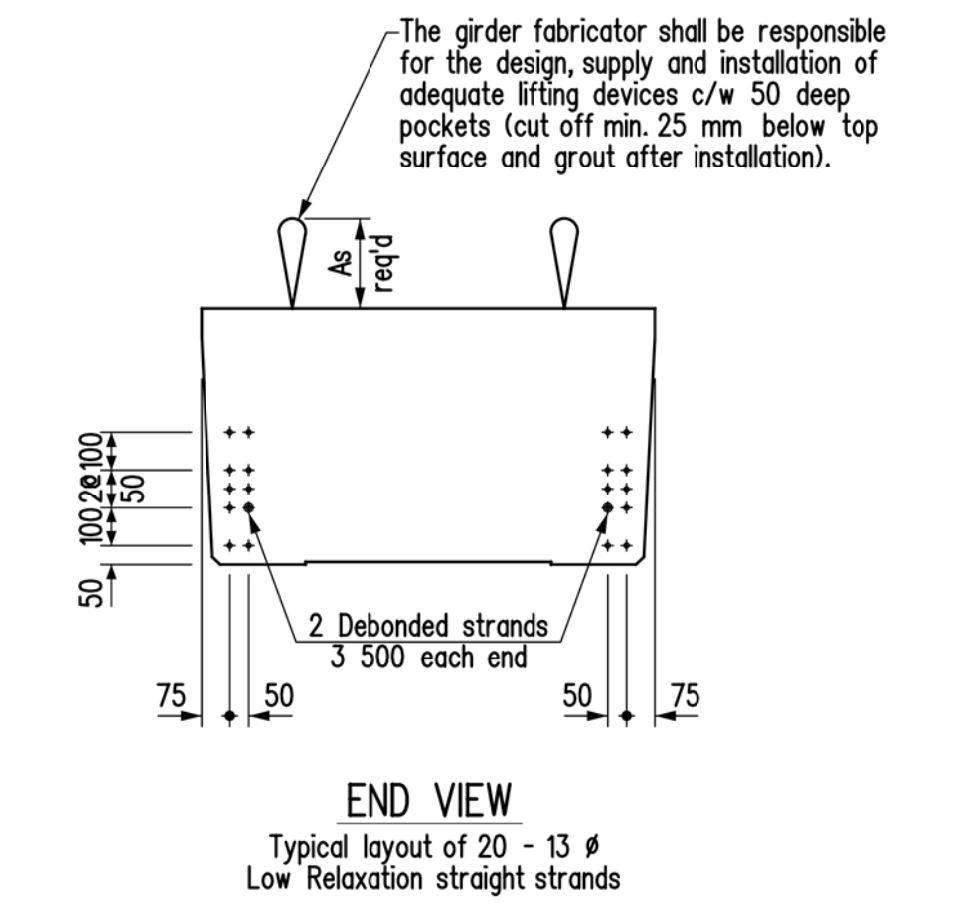
DETAIL "A" Scale 1:5  
SECTION D-D

- NOTES:
- Design in accordance with AASHTO LRFD Bridge Design Specifications, First Edition, 1994 plus 1996/1997 interim's.
  - Design Vehicular Live Load: Modified AASHTO HSS-25 AASHTO LRFD "HL-93"
  - Design distribution factor = 0.5 lanes/girder.
  - Concrete strength:  $f_c$  transfer,  $f_{ci} = 35$  MPa  
 $f_c = 45$  MPa @ 28 days,  $f_c = 45$  MPa
  - Prestressing steel: 13 mm  $\phi$  low relaxation strands  
Minimum ultimate strength,  $f_{pu} = 1860$  MPa  
Jacking force/strand,  $f_{pj} = 128.5$  kN/strand
  - Girder dimensioning tolerances: Length 3 mm  $\pm$   
Cross section 2 mm  $\pm$
  - Approximate mass per girder = 12 000 kg

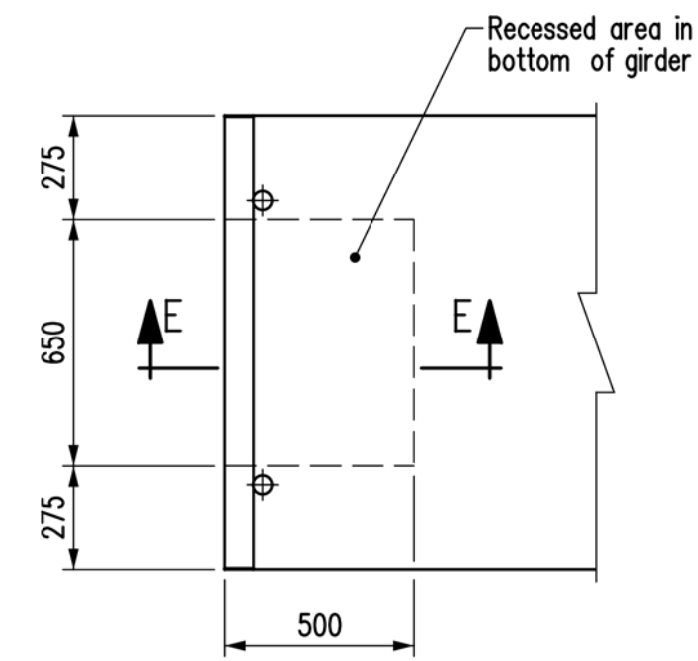
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DATE	BY	DESIGN SEAL	RECORD SEAL
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DETAILS		SCALE: Scale 1:40 SHEET No. G1	
CHECKED: [Signature]		SITE No. [Signature]	



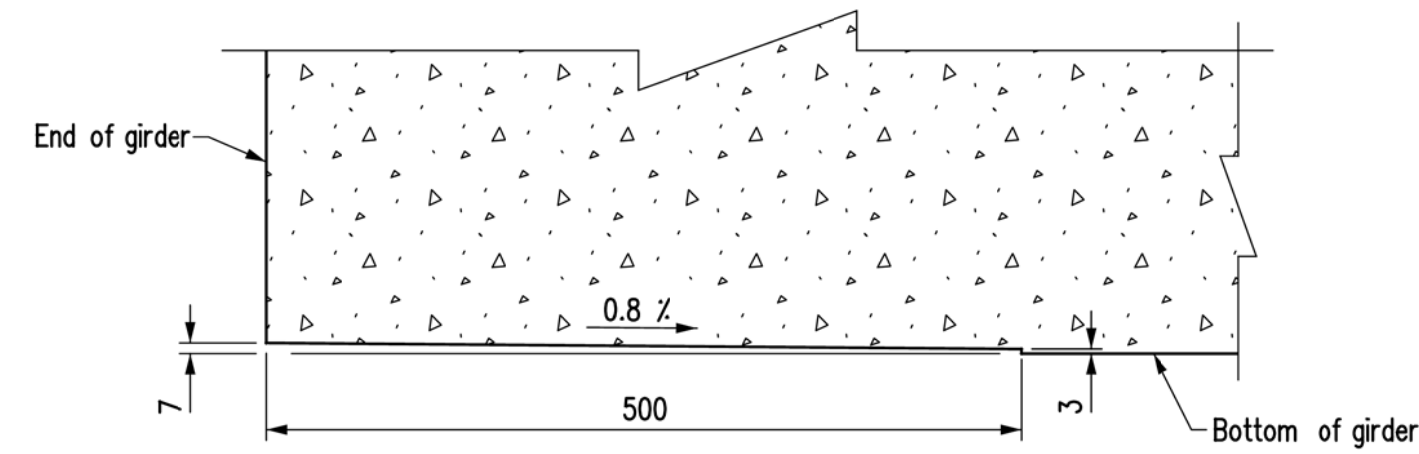
ELEVATION  
GIRDER STRAND LAYOUT



END VIEW  
Typical layout of 20 - 13 #  
Low Relaxation straight strands




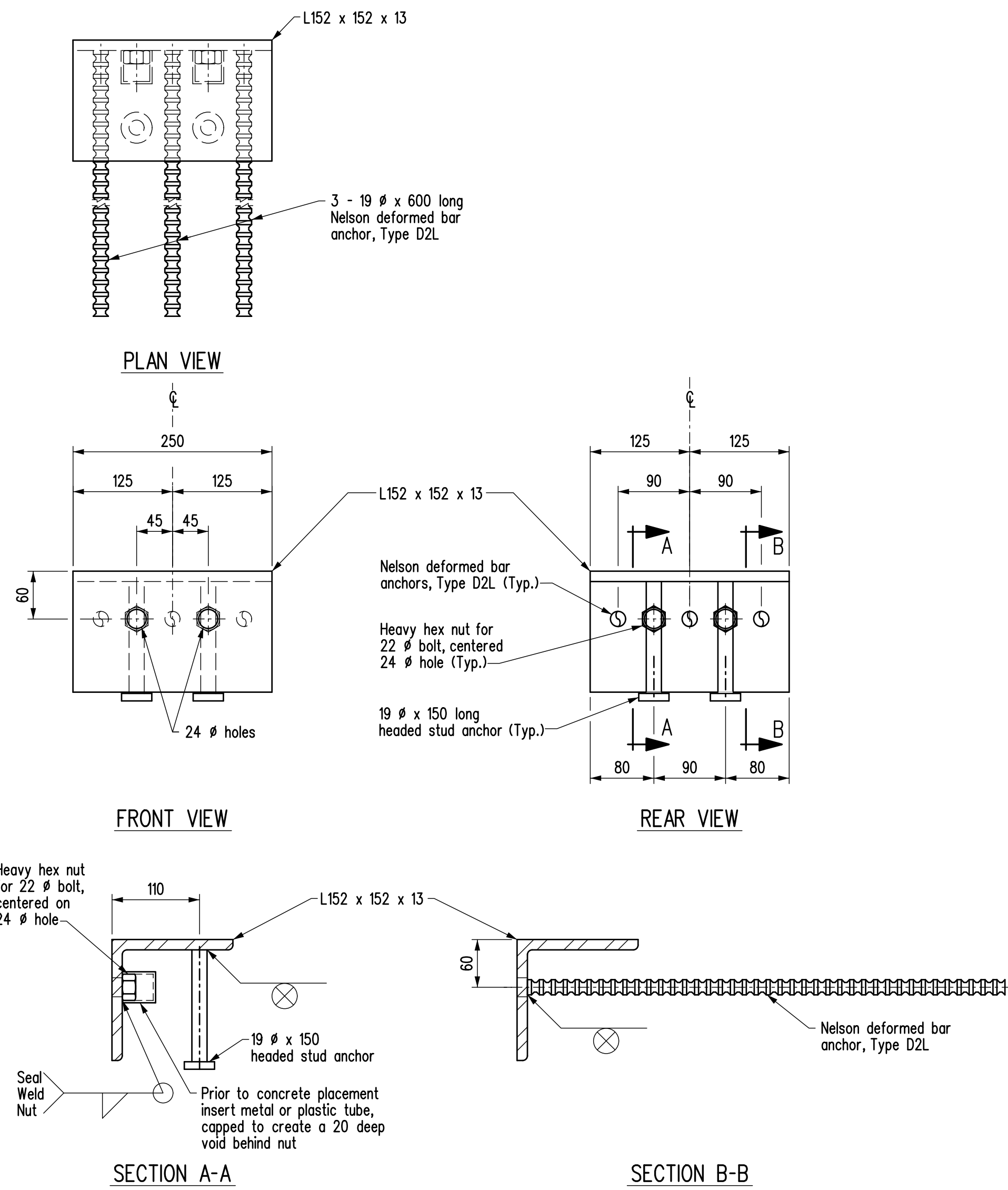
PART PLAN  
Typical at both ends of girders



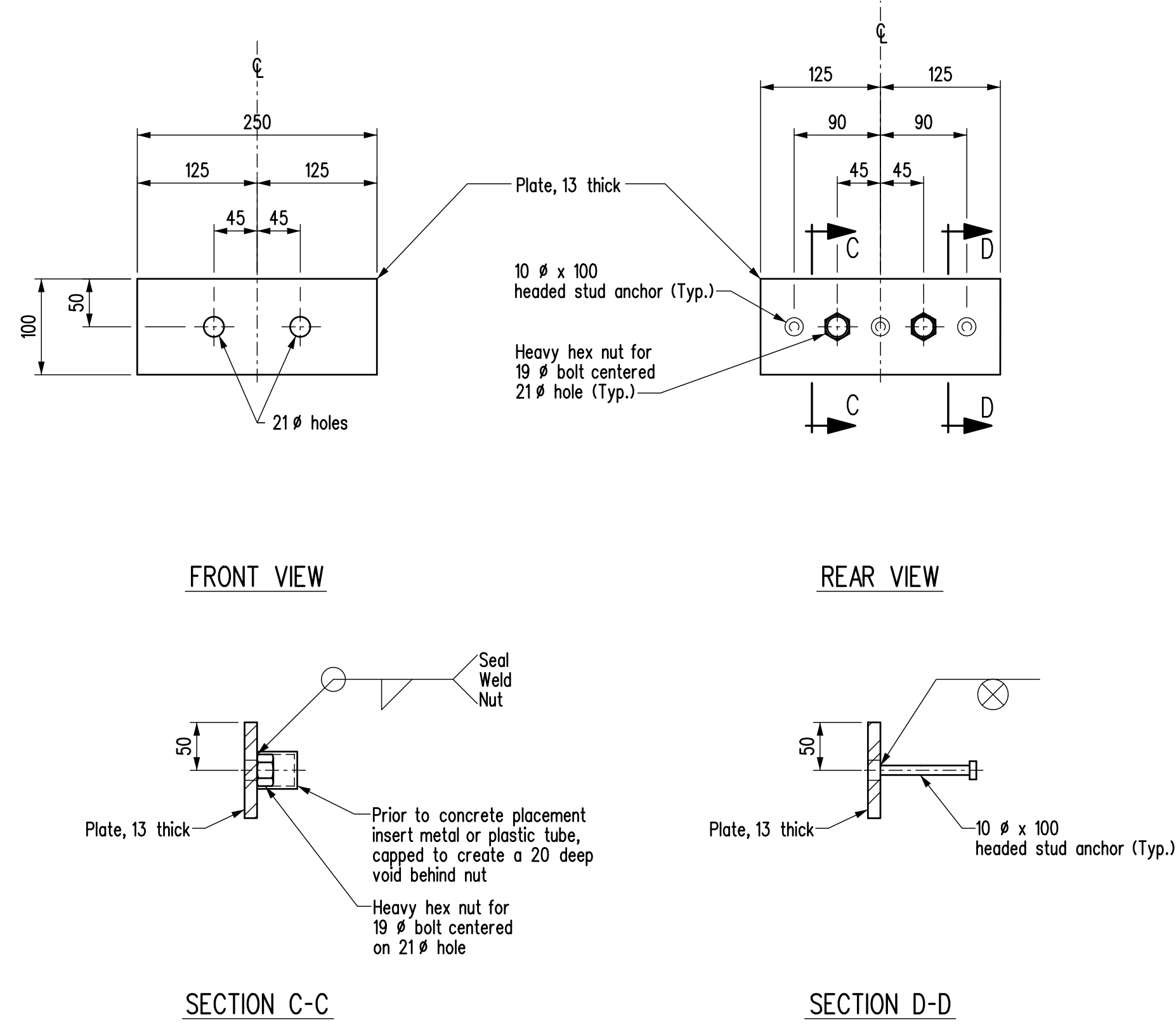
SECTION E-E  
Scale 1:5

BEARING RECESS DETAILS

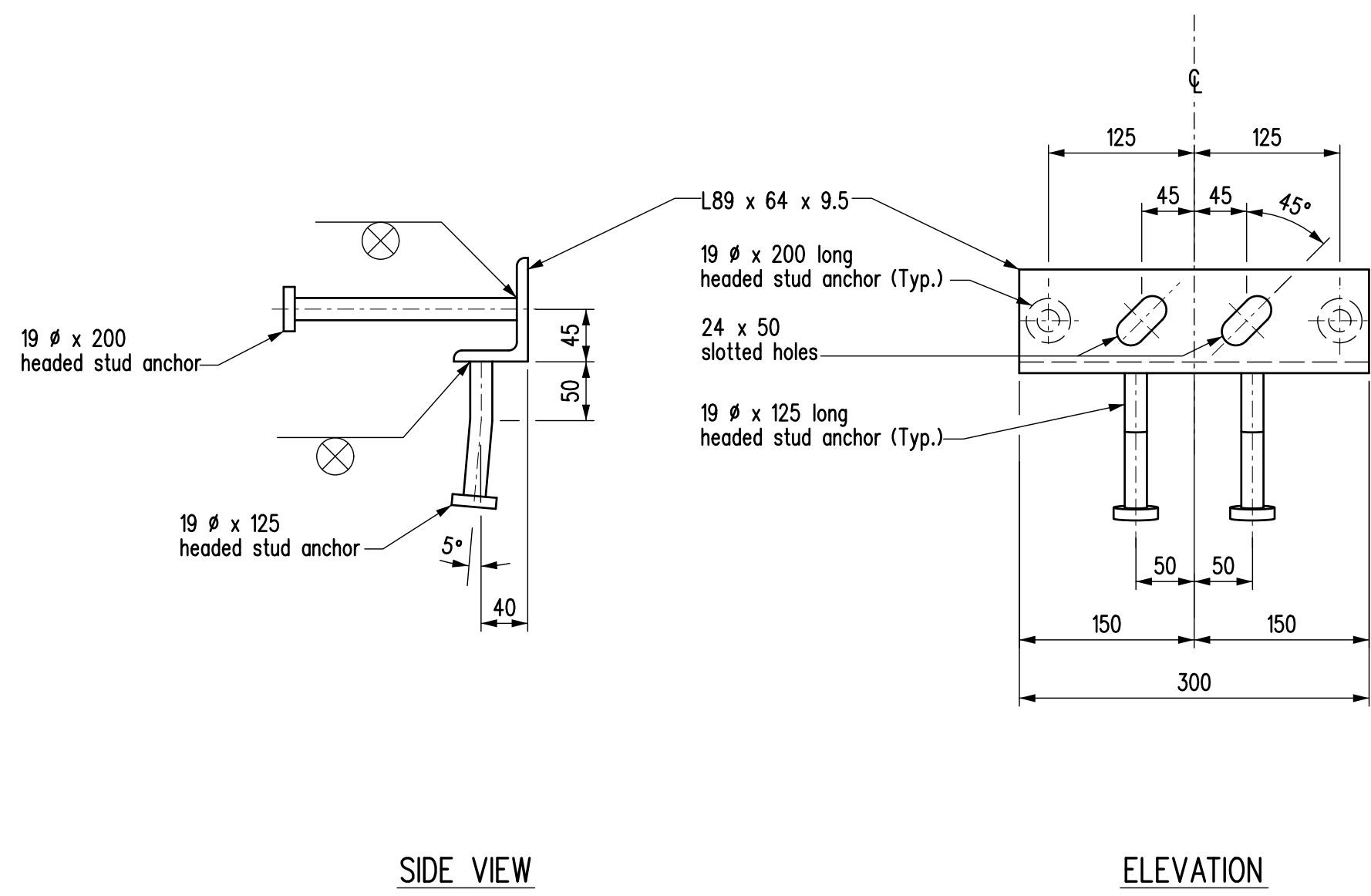
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				CHECKED:	_____	Scale 1: 20
				DETAILS	BY: _____	SHEET No. <u>  G2  </u>
				CHECKED:	_____	or as shown SITE No. _____
DATE	BY	DESCRIPTION				
DESIGN SEAL		RECORD SEAL				
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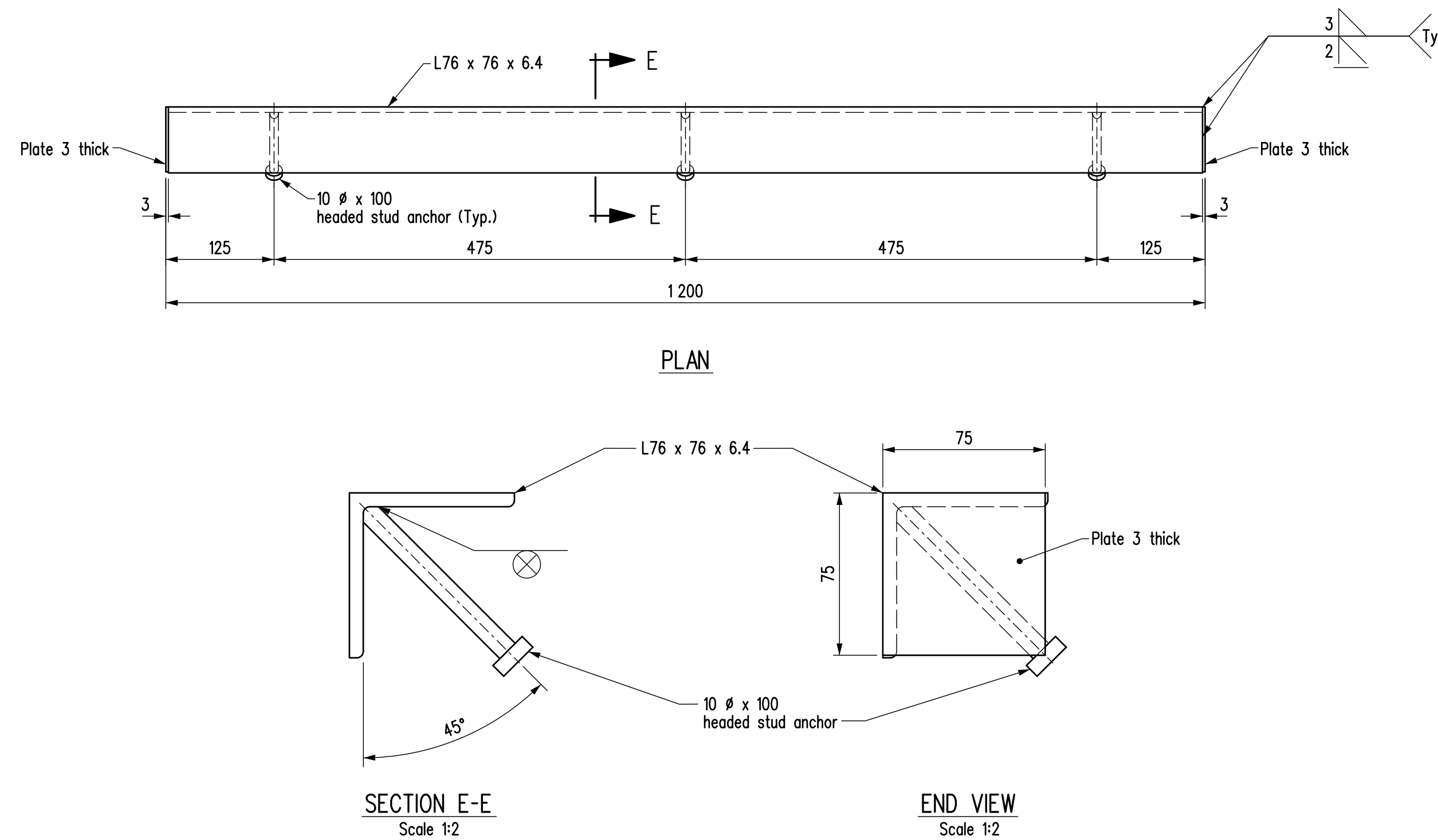
**RAILPOST ANCHOR UNIT MK. "U1"**



**RAILPOST ANCHOR UNIT MK. "U2"**



**LATERAL CONNECTION ANGLE MK. "LC1"**



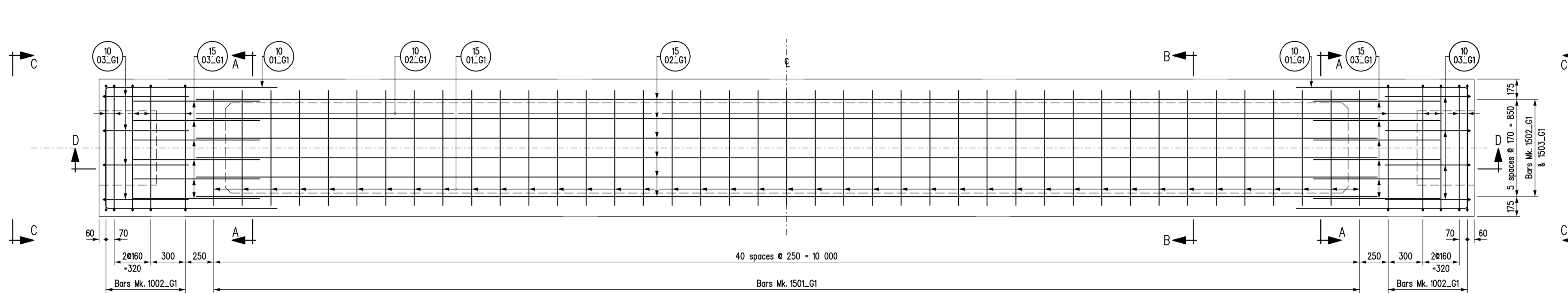
**GIRDER END ANGLE MK. "S7"**

BILL OF MISCELLANEOUS METAL						for 12 m LONG GIRDERS	9 600 ROADWAY WIDTH - 2 SPANS	Site No.
MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS		
U1	28	Railpost anchor unit	Hot dip galvanized					
		Each unit is fabricated from:						
		1 - Angle		L152x152x13	250	As detailed		
		2 - Heavy hex. Nuts		for 22 dia. bolt		Grade DH or 2H		
		2 - Studs		19 dia.	150	Headed stud anchors, ASTM A108		
		3 - Bars		for 19 dia. bolt	600	Nelson deformed bar anchors, Type D2L		
		2 - Tubes				Metal or plastic capped - As detailed		
U2	28	Railpost anchor unit	Hot dip galvanized					
		Each unit is fabricated from:						
		1 - Plate		PL 13x100	250	As detailed		
		2 - Heavy hex. Nuts		for 19 dia. bolt		Grade DH or 2H		
		3 - Studs		10 dia.	100	Headed stud anchors, ASTM A108		
		2 - Tubes				Metal or plastic capped - As detailed		
LC1	112	Lateral connection angle	Hot dip galvanized					
		Each unit is fabricated from:						
		1 - Angle		L89x64x9.5	300	As detailed		
		2 - Studs		19 dia.	200	Headed stud anchors, ASTM A108		
		2 - Studs		19 dia.	125	Headed stud anchors, ASTM A108		
S7	32	Girder end angle	Hot dip galvanized					
		Each unit is fabricated from:						
		1 - Angle		L76x76x6.4	1 194	As detailed		
		2 - Plates		PL 3x75	75	As detailed		
		3 - Studs		10 dia.	100	Headed stud anchors, ASTM A108		
	32	Ferrule loop insert	Stainless steel	for 13 dia. bolt		Richmond anchor, Type LF-W with mounting washer		
TR2	16	Threaded rod	Stainless steel	13 dia.	250	c/w hex. nut		
R27	56	A325 bolt c/w F436 hardened washer	Hot dip galvanized	22 dia.	229	Heavy hex. no nut, ASTM F3125		
R28	56	A325 bolt c/w F436 hardened washer	Hot dip galvanized	19 dia.	64	Heavy hex. no nut, ASTM F3125		

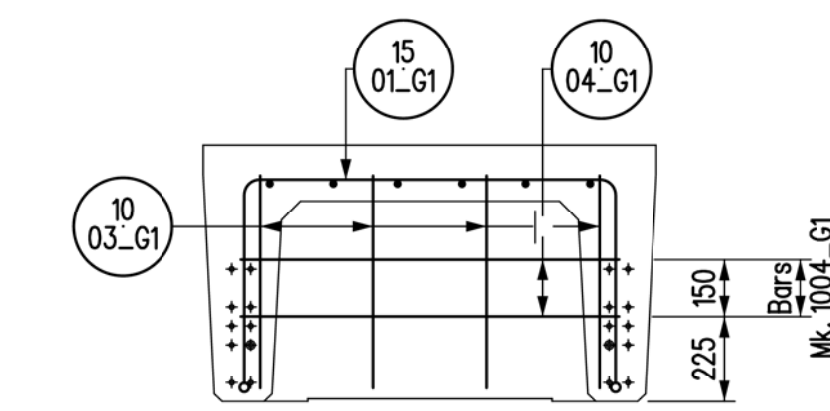
- NOTES:**
- All material in the above Bill shall be supplied by the GIRDER CONTRACTOR.
  - All structural steel shall conform to CAN/CSA G40.21-M92 Grade 300W.
  - All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m<sup>2</sup> unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.
  - Seal all welds prior to galvanizing.
  - Grade DH or 2H galvanized nuts for A325 bolts shall be overlapped to a minimum amount required for the fastener assembly in accordance with ASTM F3125. The nuts shall be lubricated with a lubricant containing a visible dye. The lubricant shall be clean and dry to the touch.
  - All bolts and inserts in the above Bill shall be Imperial thread.
  - Stainless steel shall conform to the requirements of ASTM A320, Class B8.

REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
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			or as shown SITE No.

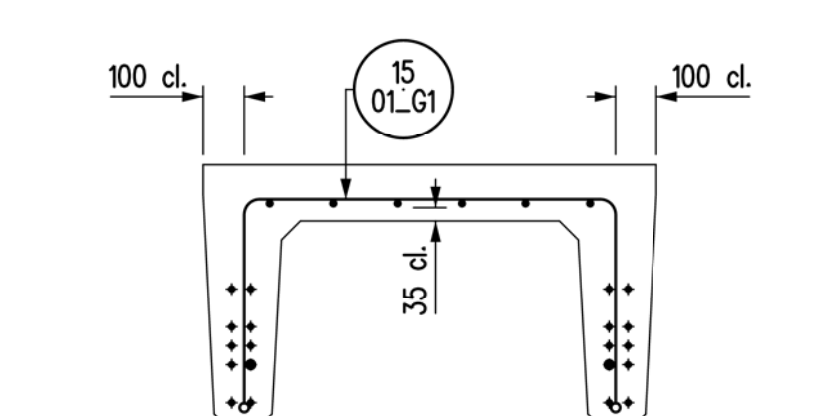




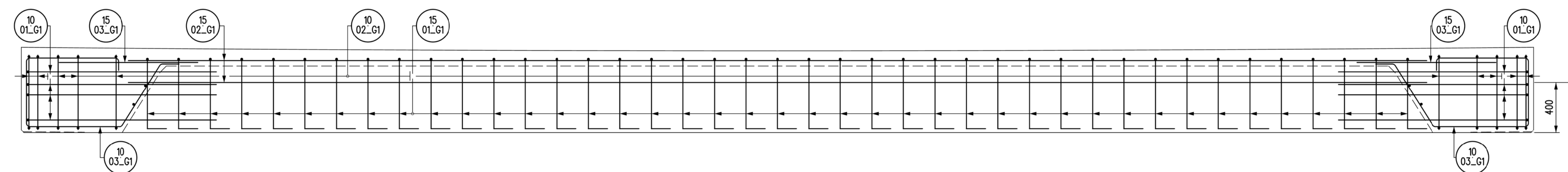
PLAN OF GIRDER



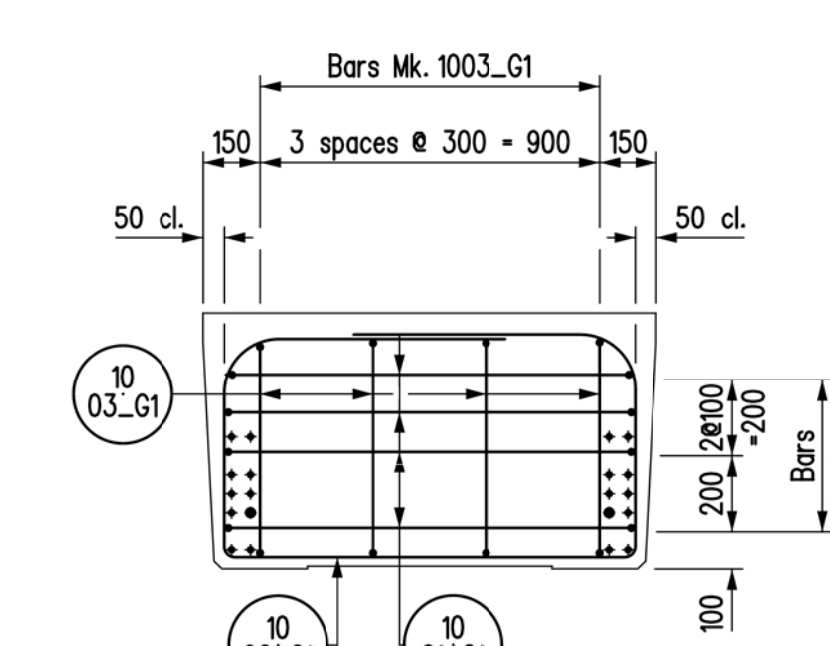
SECTION A-A



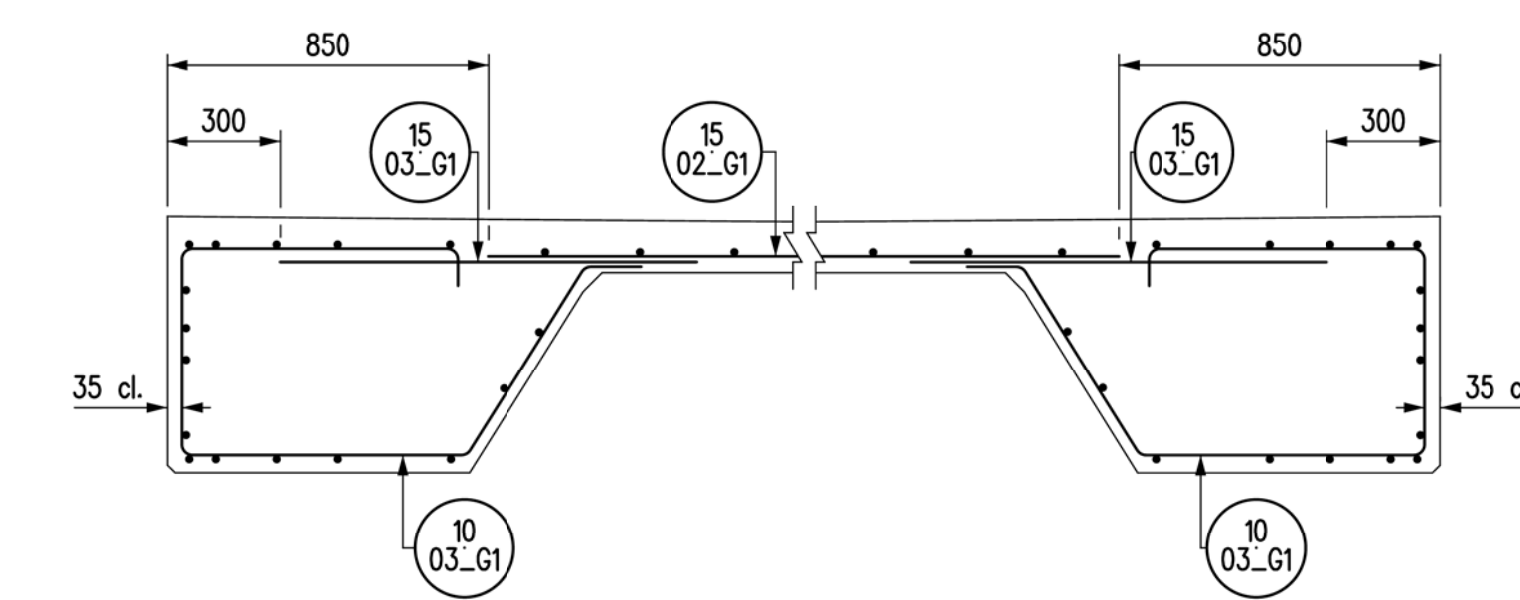
SECTION B-B



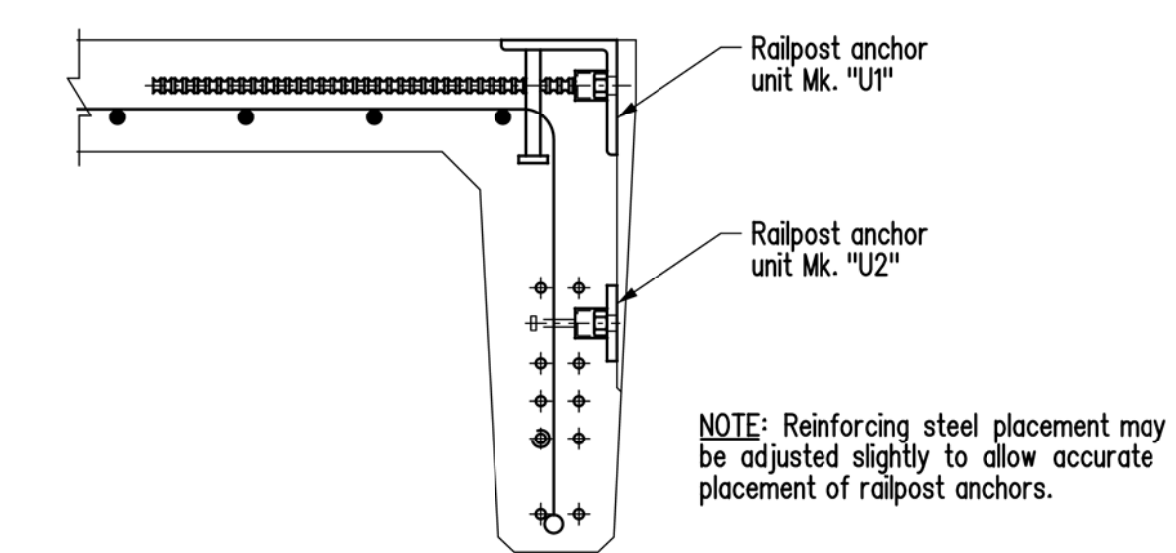
ELEVATION OF GIRDER



END VIEW C-C



PART SECTION D-D



DETAIL AT RAILPOST ANCHOR

Scale 1:10

- NOTES:
1. Concrete cover shall be 25 mm unless noted otherwise.
  2. Reinforcing details are typical for all 12 m girders unless noted otherwise.
  3. Bar Mark labels with suffix \_G1 are Exterior girders and suffix \_G2 are Interior girders. See Bill of Reinforcing Sheet No. G.

REVISIONS		PRECAST PRESTRESSED CHANNEL GIRDER DETAILS	
DATE	BY	DESCRIPTION	RELEASED FOR CONSTRUCTION BY:
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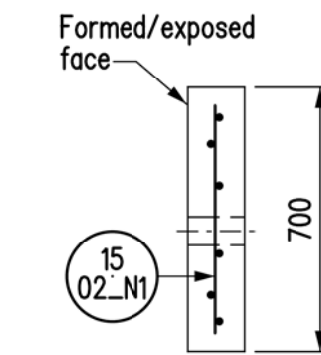
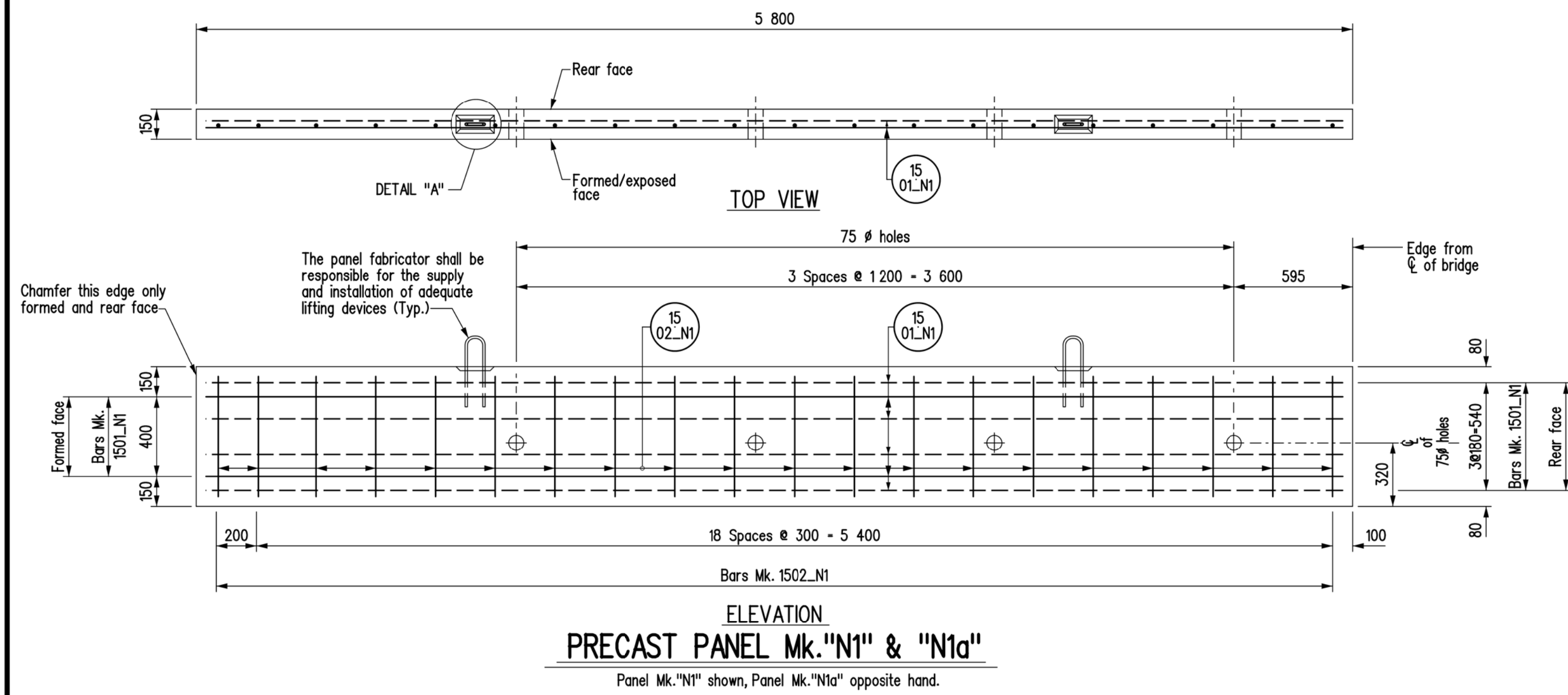


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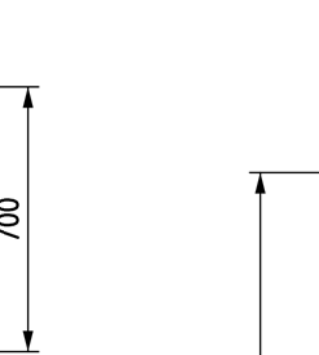
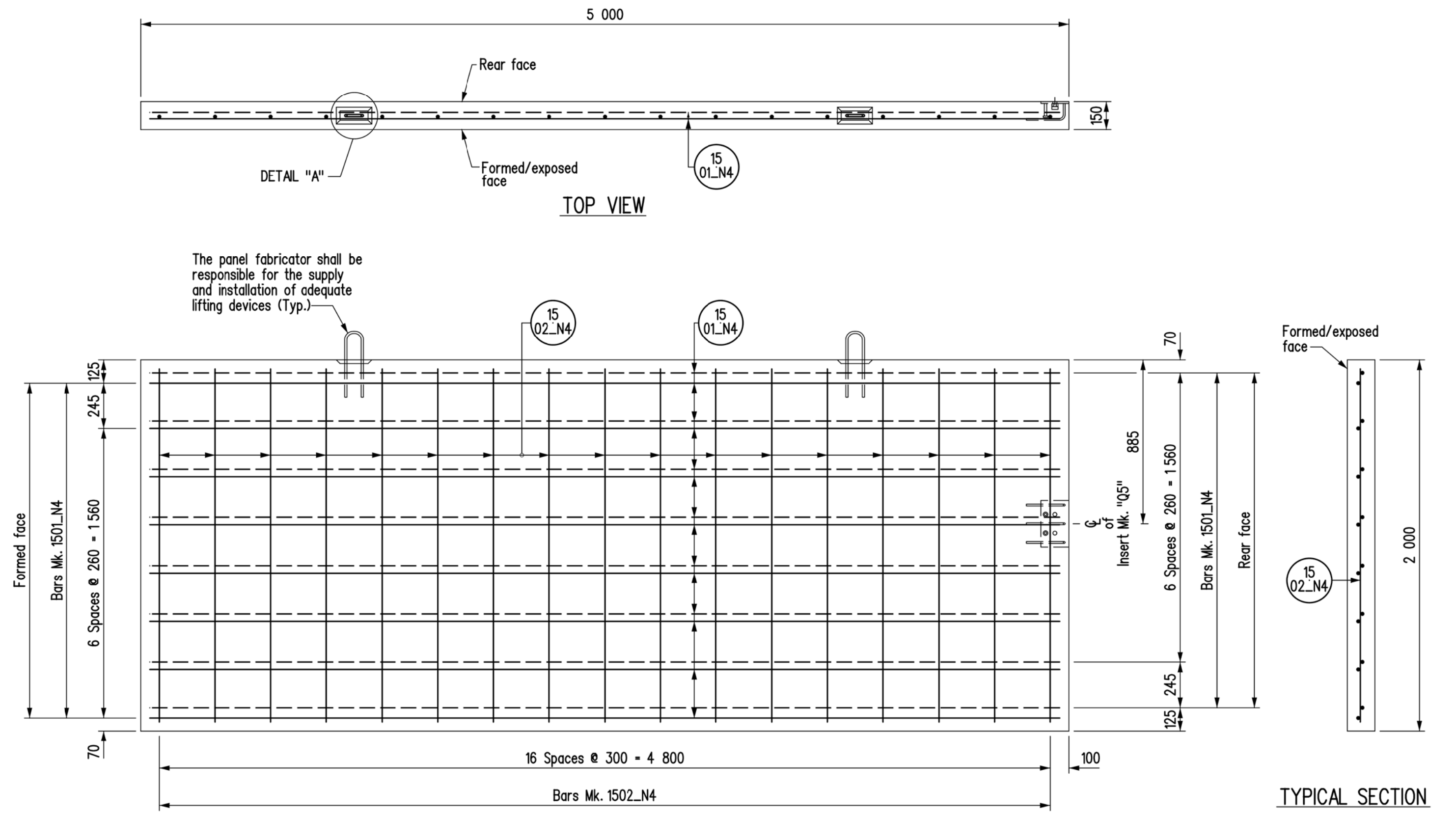
BILL OF REINFORCING STEEL - 12 M GIRDERS								SITE No.
MARK	TYPE	PIN DIAMETER	LENGTH	GIRDER TYPE	No. of GIRDERS	No. of BARS PER GIRDER	TOTAL No. of BARS PER GIRDER TYPE	BENDING DIAGRAM
1001_G1	BENT	45	4 080	G1	4	8	32	
1002_G1	BENT	45	3 660	G1	4	10	40	
1003_G1	BENT	45	2 950	G1	4	8	32	
1004_G1	STR		1 000	G1	4	4	16	
1501_G1	BENT	65	2 440	G1	4	41	164	
1502_G1	STR		10 300	G1	4	8	32	
1503_G1	STR		1 100	G1	4	12	48	
1001_G2	BENT	45	4 080	G2	12	8	96	
1002_G2	BENT	45	3 660	G2	12	10	120	
1003_G2	BENT	45	2 950	G2	12	8	96	
1004_G2	STR		1 000	G2	12	4	48	

BILL OF REINFORCING STEEL - 12 M GIRDERS								SITE No.
MARK	TYPE	PIN DIAMETER	LENGTH	GIRDER TYPE	No. of GIRDERS	No. of BARS PER GIRDER	TOTAL No. of BARS PER GIRDER TYPE	BENDING DIAGRAM
1501_G2	BENT	65	2 440	G2	12	41	492	
1502_G2	STR		10 300	G2	12	8	96	
1503_G2	STR		1 100	G2	12	12	144	
Total volume of structural concrete per exterior girder							4.94 m³	
Total volume of structural concrete per interior girder							4.93 m³	
NOTES:								
1. All dimensions given in bending diagram are out to out, except radii and extensions on 90°, 135° & 180° hooks. Extensions on 90°, 135° & 180° hooks are the "A" or "G" dimensions for standard 90°, 135° & 180° hooks referenced from the RSIC "Manual of Standard Practice". Radii are inside dimensions. All reinforcing steel bends and hooks shall conform to Clause 6.6.2 of C.S.A. A23.1-04, unless noted otherwise in the BILL OF REINFORCING STEEL.								
2. All reinforcing steel shall be deformed steel, unless noted otherwise in the BILL OF REINFORCING STEEL.								
3. All reinforcing steel shall conform to CSA G30.18-M92 "Billet Steel Bars for Concrete Reinforcement" Grade 400W, unless noted otherwise in the BILL OF REINFORCING STEEL.								
4. Like bars shall be bundled, securely tied and identified as to Mark and Site No. by appropriate means. All other items to be identified in a similar fashion.								
5. All bars shall be bent in accordance with the following detail:								

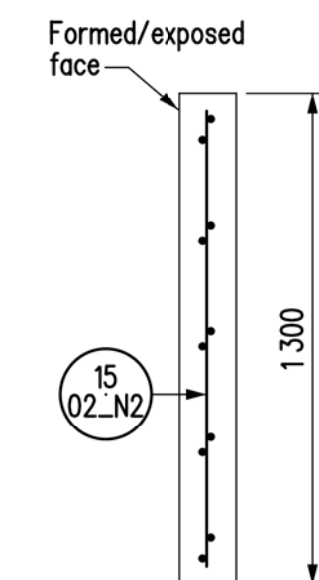
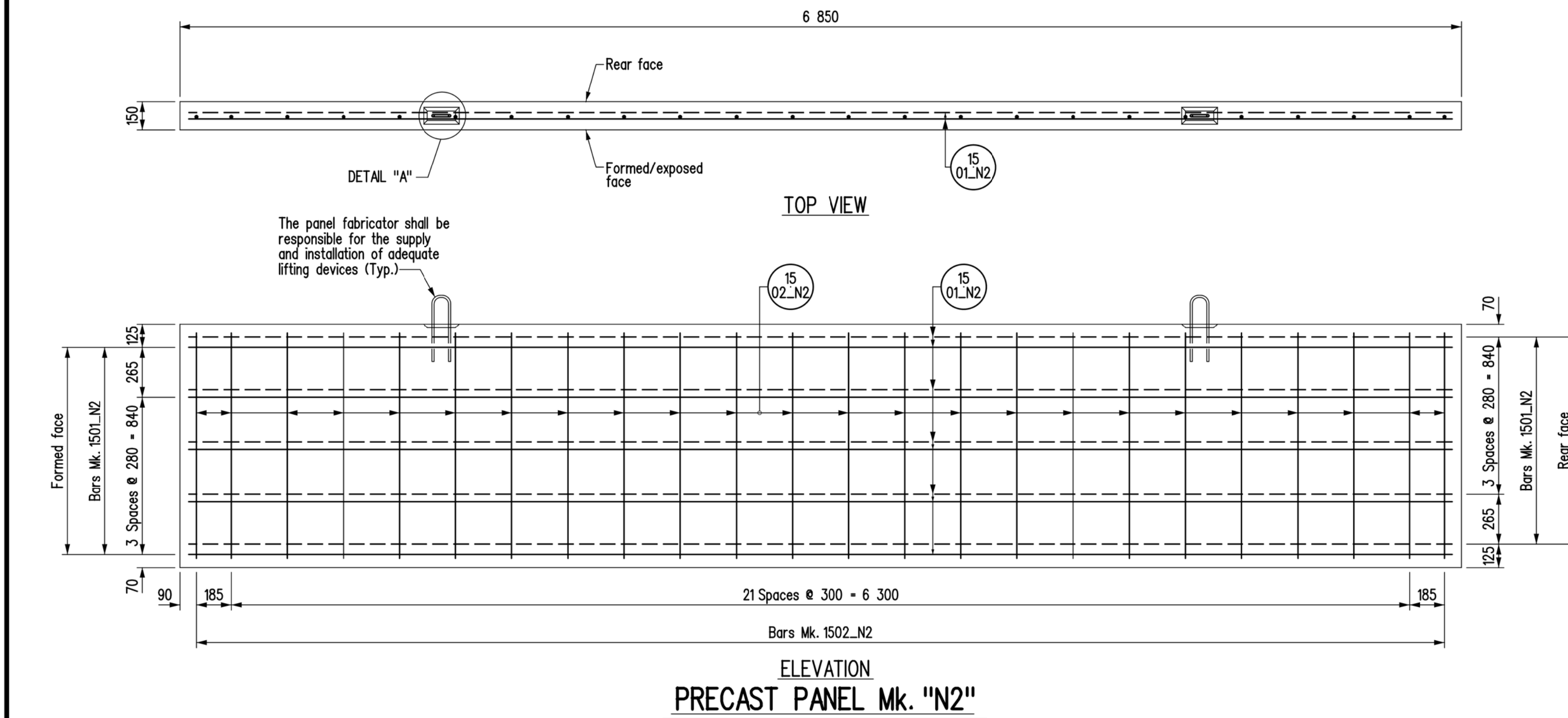
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DETAILS			
CHECKED			



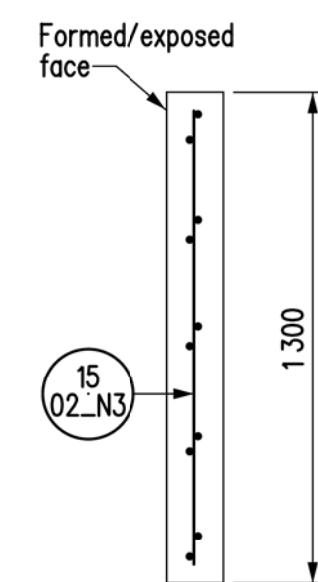
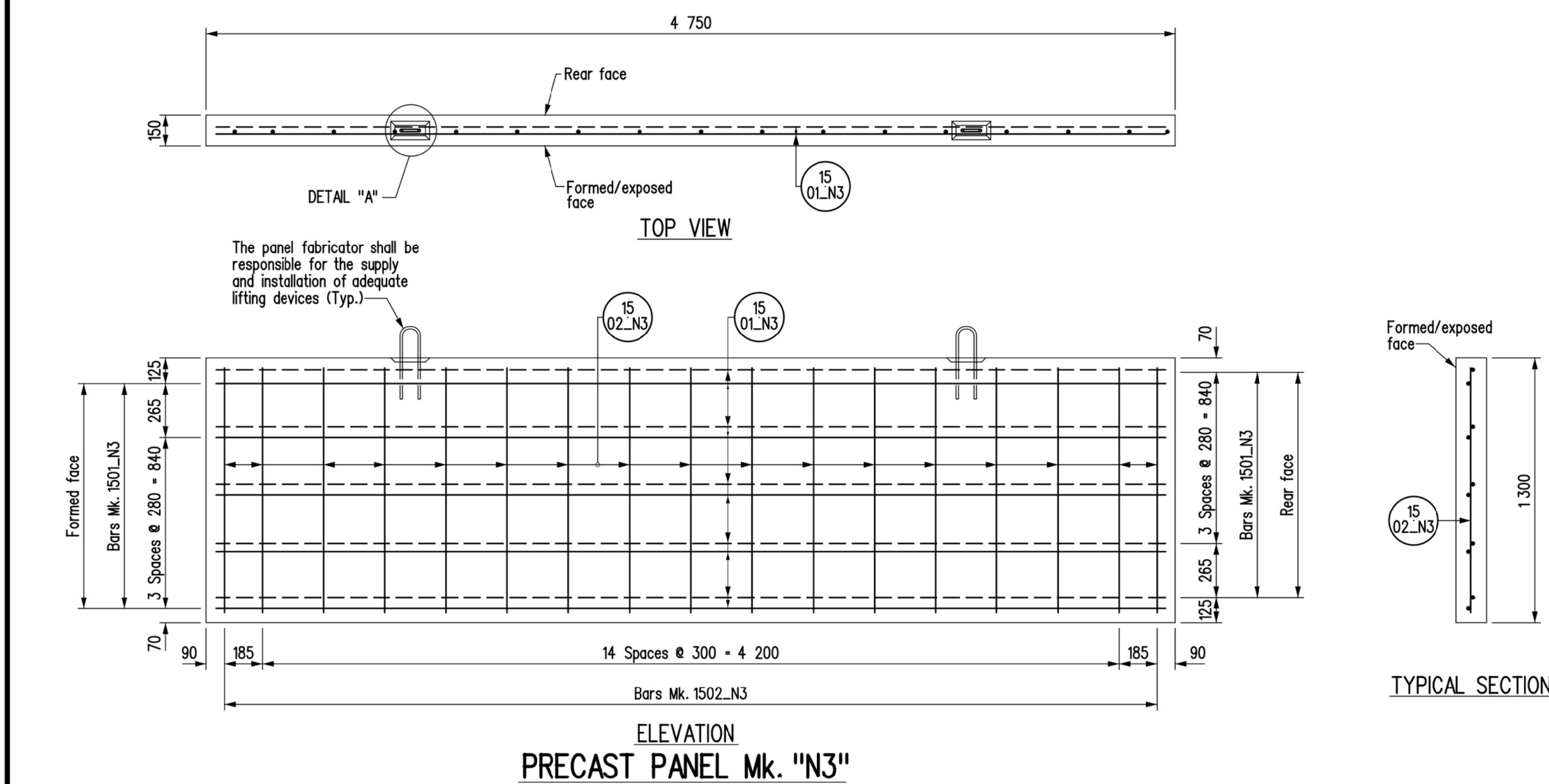
TYPICAL SECTION



TYPICAL SECTION



TYPICAL SECTION

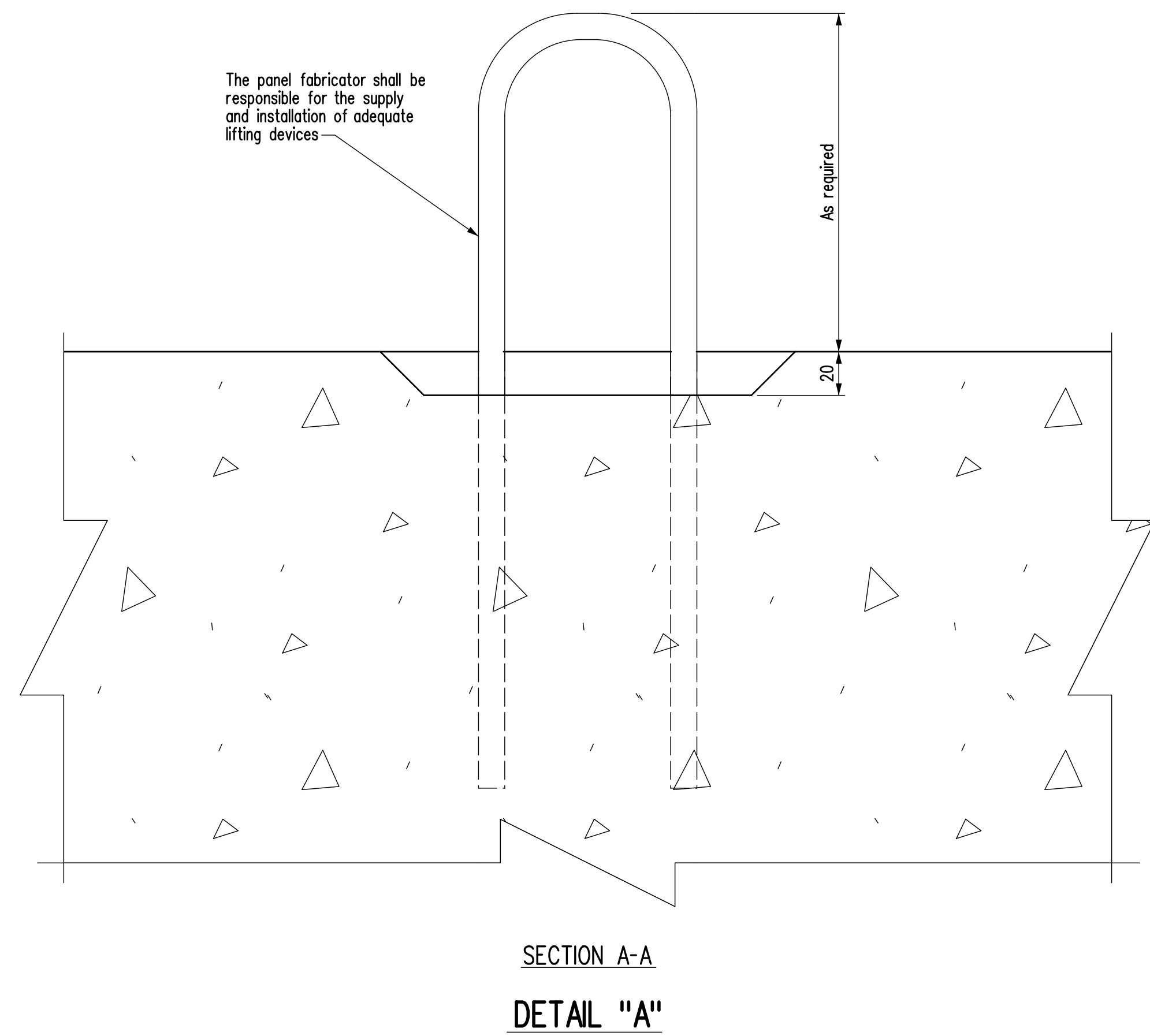
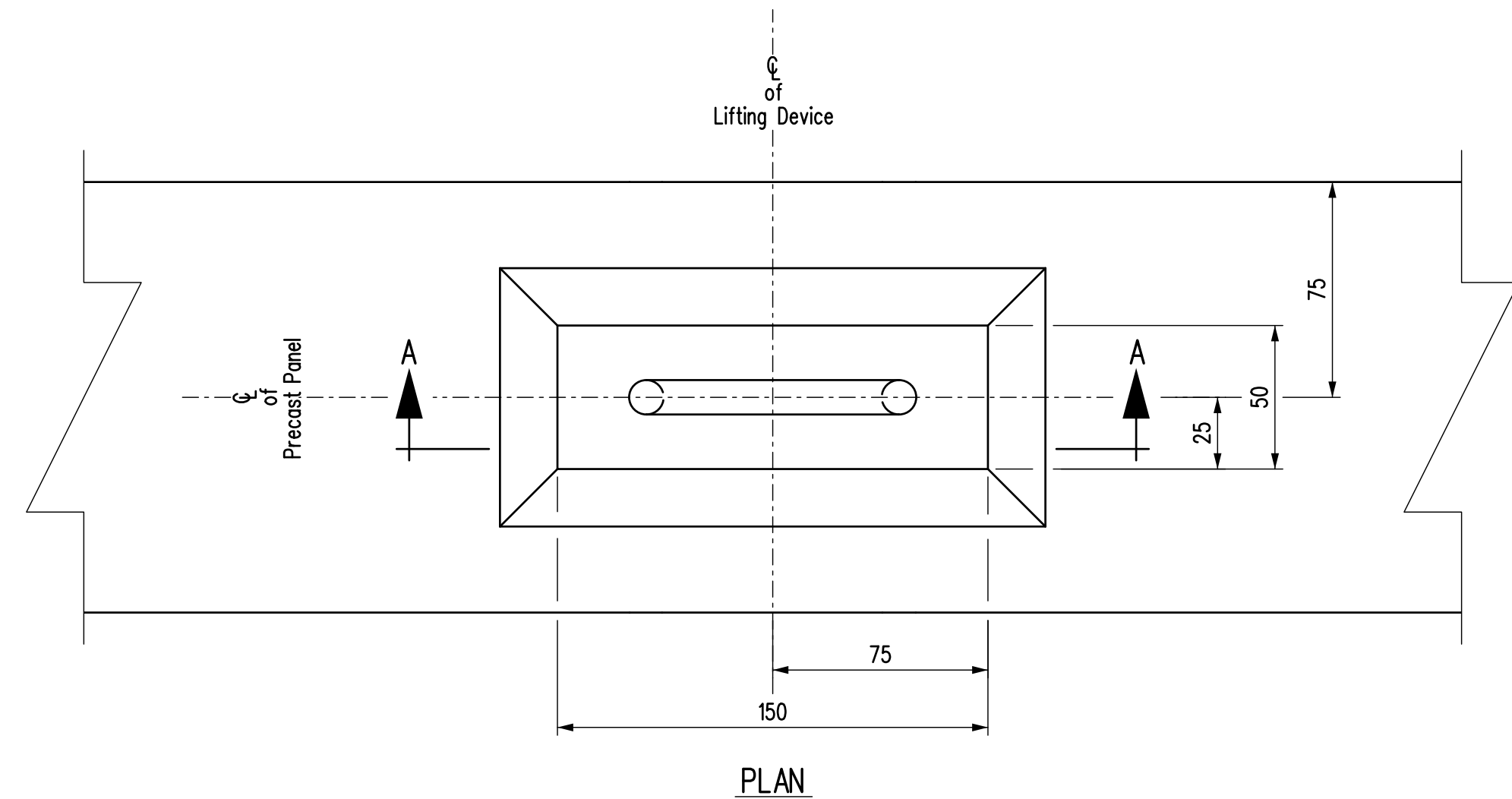


TYPICAL SECTION

**NOTES:**

- All panel exposed edges to be chamfered 20 mm except no chamfer on panels Mk. "N1" & "N1a", or if shown.
- Mark reinforcing steel location on the edges of the back face of panels after casting.
- Concrete cover shall be 50 mm unless noted otherwise.
- Formed face to be placed as exposed face during construction.
- After precast panel installation, all lifting devices to be cut-off flush and grouted as directed by Engineer.
- For DETAIL "A" see sheet No. P2.
- For BILL OF REINFORCING STEEL see Sheet No. P2.

REVISIONS		PRECAST PANEL DETAILS		
20/ / ISSUED FOR CONSTRUCTION				
DATE	BY	DESCRIPTION		
DESIGN SEAL	RECORD SEAL	<p style="text-align: center;"><b>PLACE ENGINEERS ELECTRONIC SEAL HERE</b></p>		
DESIGN				RELEASED FOR CONSTRUCTION BY: EXECUTIVE DIRECTOR OF STRUCTURES DATE
DETAILS				SCALE: 1 : 20 SHEET No. P1 or as shown SITE No.



SECTION A-A  
DETAIL "A"

**BILL OF REINFORCING  
FOR PRECAST PANELS**

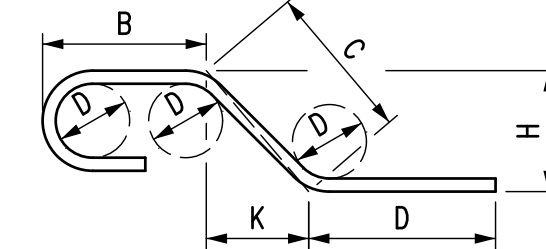
SITE No. \_\_\_\_\_

MARK	TYPE	PIN DIAMETER	LENGTH	PANEL TYPE	No. of PANELS	No. of BARS PER PANEL	TOTAL No. of BARS PER PANEL TYPE	BENDING DIAGRAM
1501_N1	STR		5 700	N1	2	6	12	BENDING DIAGRAM
1502_N1	STR		600	N1	2	20	40	
1501_N1a	STR		5 700	N1a	2	6	12	
1502_N1a	STR		600	N1a	2	20	40	
1501_N2	STR		6 750	N2	2	10	20	
1502_N2	STR		1 200	N2	2	24	48	
1501_N3	STR		4 650	N3	2	10	20	
1502_N3	STR		1 200	N3	2	17	34	
1501_N4	STR		4 900	N4	2	16	32	
1502_N4	STR		1 900	N4	2	17	34	
1501_N4a	STR		4 900	N4a	2	16	32	
1502_N4a	STR		1 900	N4a	2	17	34	

Total mass of reinforcing steel						1497.78		kg
Panel Type	N1	N1a	N2	N3	N4	N4a		
Area m <sup>2</sup> /panel	4.10	4.10	8.90	6.20	10.00	10.00		
Total area of precast Panels							86.60 m <sup>2</sup>	

**NOTES:**

- All dimensions given in bending diagram are out to out, except radii and extensions on 90°, 135° & 180° hooks. Extensions on 90°, 135° & 180° hooks are the "A" or "G" dimensions for standard 90°, 135° & 180° hooks referenced from the RSIC "Manual of Standard Practice". Radii are inside dimensions. All reinforcing steel bends and hooks shall conform to Clause 6.6.2 of C.S.A. A23.1-04, unless noted otherwise in the BILL OF REINFORCING STEEL.
- All reinforcing steel shall be deformed steel, unless noted otherwise in the BILL OF REINFORCING STEEL.
- All reinforcing steel shall conform to CSA G30.18-M92 "Billet Steel Bars for Concrete Reinforcement" Grade 400W, unless noted otherwise in the BILL OF REINFORCING STEEL.
- Like bars shall be bundled, securely tied and identified as to Mark and Site No. by appropriate means. All other items to be identified in a similar fashion.
- All bars shall be bent in accordance with the following detail:



**BILL OF MISCELLANEOUS METAL**

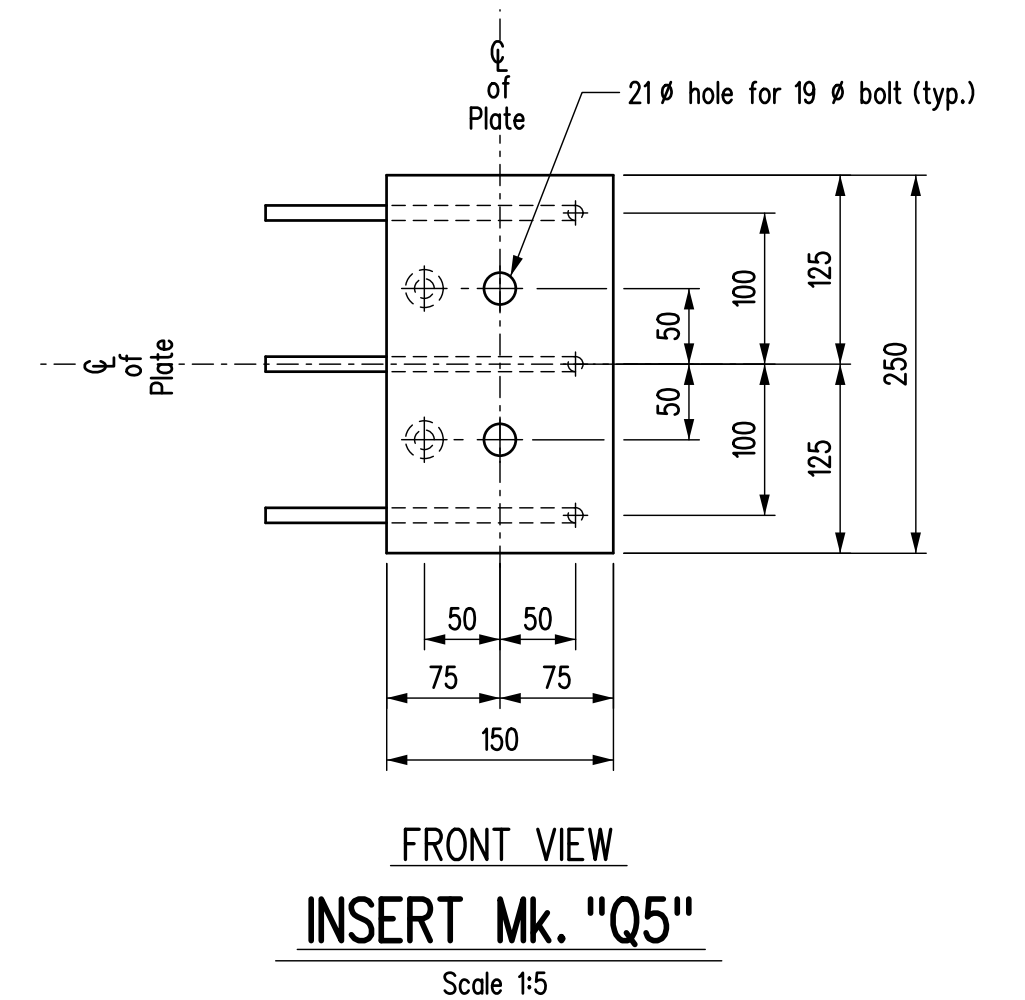
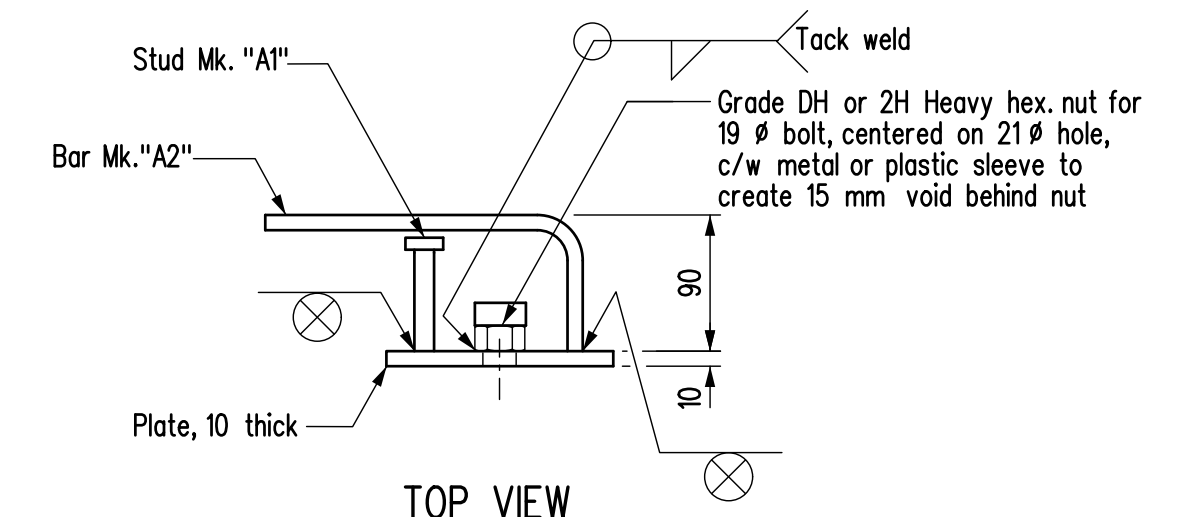
for PRECAST PANELS

Site No. \_\_\_\_\_

MARK No.	No.	DESCRIPTION	CORROSION PROTECTION	SIZE	LENGTH	REMARKS
Q5	4	Insert units	Hot dip galvanized			
		Each unit is fabricated from:				
		Steel plate		PL 10 x 150	250	As detailed
		2 - Studs Mk. "A1"		13 dia.	75	Nelson headed concrete anchors, Type H4L, Part No. 101-053-002 - As detailed
		3 - Bars Mk. "A2"		10 dia.	300	Nelson deformed bar anchors, Type D2L, Part No. 101-064-537 - As detailed
		2 - Heavy hex. nuts		for 19 dia. bolt		Grade DH or 2H heavy hex. nut, c/w metal or plastic sleeve
R34	8	A325 bolt c/w F436 hardened w washer		19 dia.	60	

**NOTES:**

- All material noted in the above Bill shall be hot dip galvanized after fabrication in accordance with CSA G164 for a minimum net retention of 610 g/m<sup>2</sup> unless otherwise stated in the specified material ASTM standards. The fabricator and galvanizer shall safeguard against embrittlement using recommended practices from applicable standards.
- Seal all welds prior to galvanizing.
- All structural steel to be CSA G40.21 Grade 300W.
- All bolts and inserts in the above Bill shall be Imperial thread.



**NOTES:**

- For location of DETAIL "A" see sheet No. P1.
- Precast panel concrete strength: f<sub>c</sub> = 35 MPa.

REVISIONS	
20___/___/____	ISSUED FOR CONSTRUCTION

DATE	BY	DESCRIPTION

DESIGN SEAL	RECORD SEAL

PLACE ENGINEERS  
ELECTRONIC SEAL  
HERE

**PRECAST PANEL DETAILS**

RELEASED FOR CONSTRUCTION	
BY:	
EXECUTIVE DIRECTOR OF STRUCTURES	DATE
SCALE: 1:2	SHEET No. P2
OR as shown	SITE No. _____



DESIGN	BY:	CHECKED:
DETAILS	BY:	CHECKED: